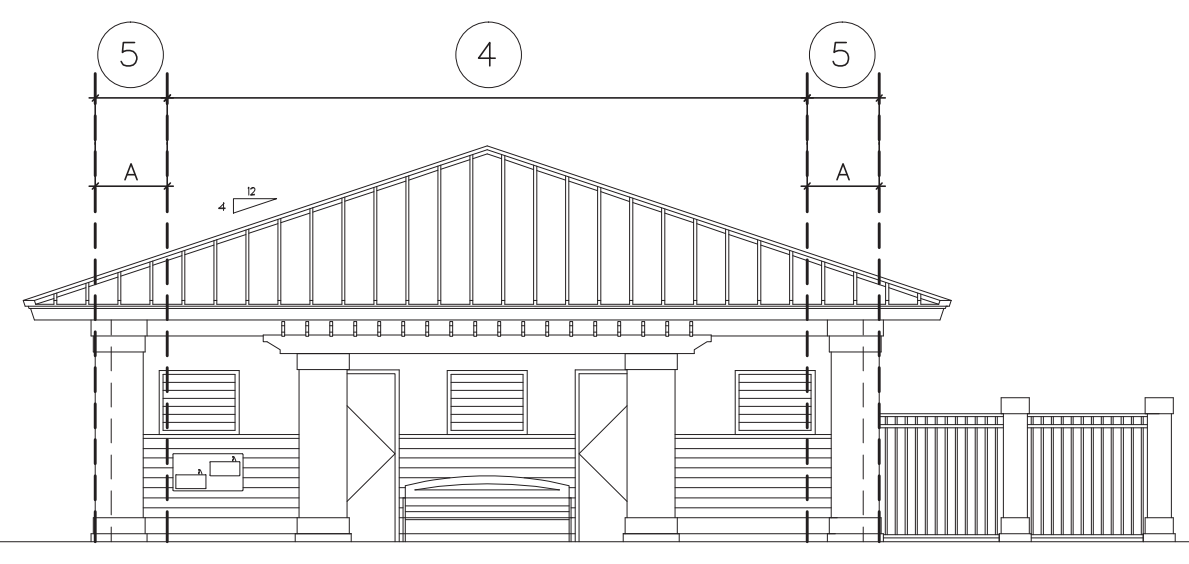




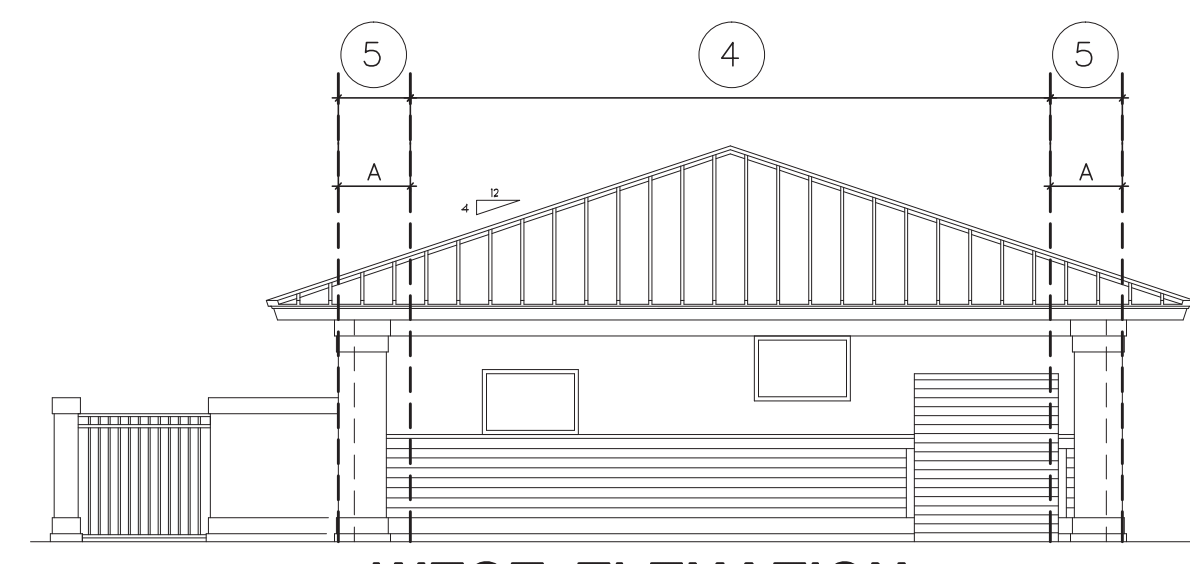
SOUTH ELEVATION
1/8"=1'-0"



EAST ELEVATION
1/8"=1'-0"



NORTH ELEVATION
1/8"=1'-0"



WEST ELEVATION
1/8"=1'-0"

STRUCTURAL NOTES

CONTRACTOR NOTE:

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. O'DONNELL, NACCARATO, MIGNAGNA & JACKSON, INC. IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

CODES AND STANDARDS

- WIND LOADS AS PER:
 - SECTION 1609.6 OF THE FLORIDA BUILDING CODE 5th EDITION (2014) WITH AN ULTIMATE WIND SPEED VULT = 170 MPH (NOMINAL WIND SPEED VASD = 132 MPH), FOR RISK CATEGORY II, EXPOSURE D AND INTERNAL PRESSURE COEFFICIENT +/- 0.18.
 - THIS BUILDING IS DESIGNED AS AN ENCLOSED BUILDING.
 - THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:
 - FLORIDA BUILDING CODE 5th EDITION (2014).
 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318/ LATEST EDITION).
 - MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315/ LATEST EDITION).
 - BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530, 530.1/ASCE 5, 6/TMS 402, 602/LATEST EDITIONS).
- ARCHITECTURAL AND MECHANICAL DRAWINGS:
 - THE STRUCTURAL DRAWINGS ARE PART OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES PROVIDE ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND COORDINATE THE INFORMATION CONTAINED IN THESE DRAWINGS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.
 - REFER TO ARCHITECTURAL, MECHANICAL OR ELECTRICAL DRAWINGS FOR ADDITIONAL OPENINGS, DEPRESSIONS, FINISHES, BOLTS SETTINGS, DRAINS, REGLETS, ETC.
 - BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL MEASUREMENTS TO PROPERLY SIZE OR FIT THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED BY THE OWNER RESULTING FROM THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.
 - DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.
 - ALL STRUCTURES HAVE BEEN DESIGNED TO RESIST THE DESIGN LOADS LISTED ONLY AS COMPLETED STRUCTURES. THE GENERAL CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT WORK IN PROGRESS UNTIL THE STRUCTURES ARE COMPLETED. THE GENERAL CONTRACTOR SHALL ALSO INSURE THAT ITS OPERATIONS AND PROCEDURES PROVIDE NO LOADING GREATER THAN THE DESIGN LOADS LISTED ON ANY MEMBER.
- SECTIONS AND DETAILS:

ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

SPECIALTY ENGINEERED PRODUCTS

- THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:
 - IN ADDITION TO THE LOADS SHOWN IN THE DESIGN LOAD SCHEDULE, THE SPECIALTY ENGINEER SHALL DESIGN FOR THE WEIGHT OF ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT AND FIXTURES, AS WELL AS CHANDELIER FIXTURES, BAR CABINETS, AND ART WORK / MOBILES.

GENERAL CONTRACTOR TO INCLUDE IN THEIR BID THE COST OF THE ABOVE NOTED SPECIALTY ENGINEERING.

FOUNDATION

- ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN STRICT ACCORDANCE WITH THE:
 - RECOMMENDATIONS ON SOILS AND FOUNDATIONS INVESTIGATION PREPARED BY AN APPROVED TESTING LABORATORY PRIOR TO FOUNDATION WORK.
- THE BUILDING SITE SHOULD BE EXCAVATED TO THE DEPTH AND EXTENT INDICATED IN THE SOILS REPORT. ALL SUBGRADES SHALL BE APPROVED IN WRITING BY THE SOILS ENGINEER PRIOR TO BACKFILLING.
- BOTTOM OF FOOTINGS ASSUMED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 2500 PSF.
- SOILS SUPPORTING ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE COMMENCING WORK, ORDERING MATERIALS, OR MOVING FORWARD IN ANY WAY. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.
- EXCAVATION & BACKFILL:
 - ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.
- CENTERLINE OF FOOTINGS: SHALL COINCIDE WITH CENTERLINE OF COLUMNS UNLESS OTHERWISE NOTED ON DRAWINGS.
- DIMENSIONS: ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS BY THE CONTRACTOR BEFORE PROCEEDING WITH THE CONSTRUCTION. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

CONCRETE

- CONCRETE ELEMENTS TO HAVE THE FOLLOWING STRENGTHS:
 - FOUNDATIONS 3000 PSI
 - SLAB-ON-GRADE 3000 PSI
 - COLUMNS 3000 PSI
 - BEAMS 3000 PSI
 - TIE BEAMS 3000 PSI
 - MASONRY GROUT 3000 PSI

ALL OTHER CONCRETE TO BE 3000 PSI UNLESS NOTED OTHERWISE.

2. ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:

- A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS
- SLUMPS SHALL BE 3" MINIMUM AND 5" MAXIMUM.
- CONCRETE SHALL HAVE 3 +/- 1.5 PERCENT AIR ENTRAINMENT.
- ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.55.
- JOBSITE WATER SHALL NOT BE ADDED.
- CEMENT SHALL CONFORM WITH ASTM C150 TYPE 1. SLAG, ASTM C989 SHALL BE LIMITED TO 50% (BY WEIGHT) OF CEMENTITIOUS MATERIAL AND FLY ASH, ASTM C918, CLASS F, SHALL BE LIMITED TO 25% (BY WEIGHT) OF CEMENTITIOUS MATERIAL.

3. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE ACI BUILDING CODE (ACI 318/ LATEST EDITION), THE ACI DETAILING MANUAL (ACI 315/ 1994 EDITION), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301/ LATEST EDITION).

- CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.
- WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A 185, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.
- REQUIREMENTS:
 - ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A 615 GRADE 60.
 - WWF SHALL COMPLY WITH ASTM A 185.
 - PROVIDE 10 MIL VAPOR BARRIER COMPLIANT WITH ASTM E1745 BELOW ALL SLAB ON GRADE.

7. REINFORCING BARS:

- AT CORNERS OF CONCRETE WALLS, BEAMS AND CONTINUOUS WALL FOOTINGS, PROVIDE 1#-#5 OR MATCHING HORIZONTAL BARS X 5'-0" BENT BAR FOR EACH HORIZONTAL BAR SCHEDULED AT EACH FACE.
- WHERE COLUMNS ARE AN INTEGRAL PART OF CONCRETE WALLS, WALL REINFORCEMENT SHALL BE CONTINUOUS THRU THE COLUMNS.
- ALL HOOKS SHOWN IN REINFORCEMENT SHALL BE ACI RECOMMENDED HOOKS UNLESS OTHERWISE NOTED.
- CONCRETE LINTELS:
 - DROP BOTTOM OF BEAM OR SLAB AT WINDOWS, DOORS AND MASONRY OPENINGS AS REQUIRED TO PROVIDE A CONCRETE CLOSURE BETWEEN THE BOTTOM OF THE BEAM AND WINDOW AND/OR DOOR HEADER OR PROVIDE A PRECAST CONCRETE LINTEL BY CASTCRETE IF NOT NEXT TO A POURED CONCRETE COLUMN.
 - MAXIMUM DROP SHALL BE 16" (TWO BLOCK COURSES) AND SPAN EQUAL TO MASONRY OPENING WIDTH. PROVIDE 2 #5 AT BOTTOM OF DROP INCLUDING #3 TIES @ 24" O.C. EXTENDING TO TOP OF BEAM REINFORCING. IF THE LINTEL EXCEEDS THE ABOVE LIMIT OF DROP, A SEPARATED LINTEL SHALL BE PROVIDED AS FOLLOWS:
 - OPENING LESS THAN 6'0" WIDE 8" X 8" W/2 #5 BOTTOM BARS.
 - OPENING BETWEEN 6'0" AND 12'0" WIDE 8" X 12" W/2 #6 BOTTOM BARS.

MASONRY

- MASONRY UNITS SHALL BE
 - LOAD BEARING ASTM C90
 - TYPE II NON-MOISTURE CONTROLLED
 - NORMAL WEIGHT
 - ALL CMU SHALL BE LAID IN A FULL BED OF MORTAR IN RUNNING BOND (U.N.O.).
- THE COMPRESSIVE STRENGTH OF MASONRY (F'M) SHALL BE 1,500 PSI AS CALCULATED IN ACCORDANCE WITH ASTM C1314.
- ALL MORTAR SHALL BE IN ACCORDANCE WITH ASTM SPECIFICATION C270.
 - FROM FIELD OBTAINED TEST CUBES. (MIN. OF TWO)
 - GROUT SHALL BE A HIGH SLUMP MIX
 - IN ACCORDANCE WITH ASTM SPECIFICATION C476
 - HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI
 - FROM FIELD OBTAINED TEST CUBES. (MIN. OF TWO)
- ALL CONCRETE MASONRY BEARING AND SHEAR WALLS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENT FOR MASONRY STRUCTURES" (ACI 530/ASCE 5/TSM 402) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TSM 602)/ LATEST EDITIONS.
- PROVIDE 8" X 8" MASONRY BEAM WITH 2 #5 CONT. AT EVERY WINDOW SILL. EXTEND BEAM 8" BEYOND EDGE OF OPENING.
- PROVIDE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (9 GA.) AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS. PROVIDE DOVE TAIL SLOT ANCHORS AT CONCRETE COLUMNS.

FOR JOINT REINFORCEMENT, WALL TIES, ANCHORS AND INSERTS, APPLY A MINIMUM COAT OF 1.5 OUNCES PER SQUARE FOOT (PSF) (45#/G/M2) COMPLY WITH THE REQUIREMENTS OF ASTM A153, CLASS B.

7. PROVIDE CONTROL JOINTS IN MASONRY WALLS AT A SPACING OF 30' + O.C. AND ALIGN WITH ARCHITECTURAL CONTROL JOINTS.

9. MINIMUM LAP SPLICES FOR REINFORCED CMU PER 2010 FBC SECTION 2107:

BAR SIZE	#4	#5	#6	#7	#8
BAR SIZE	24"	30"	36"	42"	48"
A. LAP SPLICES SHALL OCCUR DIRECTLY ABOVE FOOTINGS AND SLABS. NO SPLICES ARE ALLOWED AT MID-HEIGHT OF WALL.					
B. LAP SPLICES THAT OCCUR AT CANTILEVERED WALLS SUCH AS: PARAPETS, RETAINING WALLS, ETC. SHALL HAVE LAP SPLICE LENGTHS INCREASED BY 50% TO 72 BAR DIAMETERS.					

10. MASONRY LINTELS:

- A PRECAST CONCRETE LINTEL BY CASTCRETE SHALL BE PROVIDED OVER ALL MASONRY WALL OPENINGS. THE LINTEL SHALL BE FULLY GROUTED.
- LINTELS TO HAVE 4" MINIMUM BEARING AT EACH END.

C. SHORE PRECAST LINTEL PER MANUFACTURER'S INSTRUCTIONS.

PREFABRICATED METAL ROOF TRUSSES

1. TRUSS SUBMITTALS AND DESIGN LOADS:

- SHOP DRAWINGS AND DESIGN COMPUTATIONS:
 - ENGAGE THE SERVICES OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA TO PREPARE COMPLETE SHOP DRAWINGS AND STRUCTURAL DESIGN COMPUTATIONS FOR WORK OF THIS SECTION. DRAWINGS SHALL BEAR THE ENGINEER'S PROFESSIONAL SEAL.
 - THE SHOP DRAWINGS SHALL SHOW ALL PERTINENT DETAILS OF CONSTRUCTION, INSTALLATION, AND ANCHORAGE OF THE LIGHT GAUGE STEEL FRAMING WORK.
 - THE STRUCTURAL DESIGN COMPUTATIONS SHALL PROVIDE A COMPLETE STRUCTURAL ANALYSIS OF ALL TYPICAL AND SPECIAL CONDITIONS OF CONSTRUCTION, AND SHALL CERTIFY CONFORMANCE TO THE GOVERNING LAWS AND BUILDING CODES.
 - STRICTLY FOLLOW L.G.S.E.A.'S "FIELD INSTALLATION GUIDE FOR COLD-FORMED STEEL ROOF TRUSSES" FOR THE PROPER STORAGE, HANDLING AND BRACING REQUIREMENTS.

B. METAL TRUSS MANUFACTURER TO DESIGN BOTTOM CHORDS OF METAL TRUSSES FOR A MINIMUM OF 10 PSF LIVE LOAD. (BOTTOM CHORDS OF METAL ATTIC TRUSSES TO BE DESIGNED FOR 30 PSF MINIMUM LIVE LOAD. IN ADDITION TO THE LOADS SHOWN IN THE DESIGN LOAD SCHEDULE, THE SPECIALTY ENGINEER SHALL DESIGN FOR THE WEIGHT OF ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT AND FIXTURES, AS WELL AS CHANDELIER FIXTURES, BAR CABINETS, AND ART WORK / MOBILES.

C. SAMPLES: SUBMIT REPRESENTATIVE SAMPLES OF ALL LIGHT GAUGE STEEL FRAMING COMPONENTS TO ARCHITECT FOR APPROVAL.

D. PRODUCT DATA: SUBMIT MANUFACTURER'S PRODUCT DATA FOR ALL COMPONENTS TO BE USED IN THE CONSTRUCTION AND ANCHORING OF THE LIGHT GAUGE STEEL FRAMING. INCLUDE SPECIFICATIONS, INSTALLATION INSTRUCTIONS, AND DATA SUBSTANTIATING THAT THE MATERIALS COMPLY WITH SPECIFIED REQUIREMENTS.

2. FRAMING COMPONENTS FOR METAL ROOF TRUSSES:

- STUDS SHALL BE 16-GAGE OR HEAVIER PUNCHED C STUDS. STUDS 2. SYMMETRIC SHAPES SHALL BE 18-GAGE OR HEAVIER. MEMBERS SHALL BE MANUFACTURED FROM STEEL MEETING THE REQUIREMENTS OF ASTM A653, GRADE D WITH A MINIMUM YIELD STRENGTH OF 50,000 PSI. MEMBERS SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A653 G60 COATING DESIGNATION. MAXIMUM SPACING OF TRUSSES SHALL BE 4'-0" O.C.
- TRACKS SHALL BE 18 GAUGE OR HEAVIER UN-PUNCHED TRACKS MANUFACTURED OF COMMERCIAL QUALITY STEEL SHEET MEETING THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD STRENGTH OF 50,000 PSI. TRACKS SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A653, G60 COATING DESIGNATION. PROVIDE SPECIAL SHAPED TRACKS WITH ONE 4 INCH HIGH LEG WHERE INDICATED ON DRAWINGS.
- BRIDGING SHALL BE MANUFACTURER'S RECOMMENDED TYPE AS REQUIRED TO MEET THE DESIGN CRITERIA SET FORTH ABOVE.
- ATTACHMENT ANGLES, CLOSURE ANGLES, AND OTHER MISCELLANEOUS COMPONENTS SHALL BE MANUFACTURED OF COMMERCIAL QUALITY STEEL SHEET MEETING THE REQUIREMENTS OF ASTM A653 WITH A MINIMUM YIELD STRENGTH OF 50,000 PSI AND SHALL BE FORMED TO PROFILES AS REQUIRED. ALL COMPONENTS SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A653, G60 COATING DESIGNATION.

E. SUBCONTRACTOR SHALL COORDINATE TRUSS LOCATIONS WITH MECHANICAL TRADES.

3. FIELD TOUCH UP:

- TOUCH UP ALL FIELD WELDS AND ABRASIONS OF GALVANIZED MATERIALS WITH ZINC RICH PAINT IN ACCORDANCE WITH ASTM A 780, ANNEX A2.
- TOUCH UP WORK SHALL BE COMPLETED PRIOR TO ATTACHMENT OF THE WORK OF ANY OTHER SECTIONS TO THE LIGHT GAUGE STEEL FRAMING.

SHOP DRAWINGS

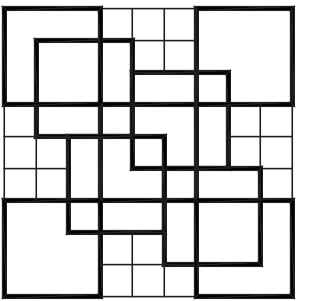
- THE SHOP DRAWINGS SHALL BE SUBMITTED IN COMPLETE PACKAGES FOR THE FOLLOWING:
 - CONCRETE MIX DESIGNS
 - CONCRETE REINFORCING STEEL AND WELDED WIRE FABRIC
 - CONCRETE MASONRY UNIT SUBMITTALS AND OTHER MASONRY ACCESSORIES
 - PRE-ENGINEERED LIGHT-GAGE METAL TRUSSES
- PRE-ENGINEERED ITEMS SHALL BE SUBMITTED SIGNED AND SEALED BY A SPECIALTY ENGINEER REGISTERED IN THE STATE OF FLORIDA.

ULTIMATE (Wx1)						
COMPONENT & CLADDING WIND DESIGN PRESSURES						
PRESSURES BASED ON V _w	ROOF WIND LOADS					WALL WIND LOADS
	ROOF AREA (10 SF)					WALL AREA (10 SF)
Kd IS INCLUDED	1	2	3	2 (OH)	3 (OH)	4
PRESSURE (PSF)	44.06	44.06	44.06	32.39	32.39	76.45
SUCTION (PSF)	-69.97	-121.80	-121.80	-142.53	-239.71	-82.93

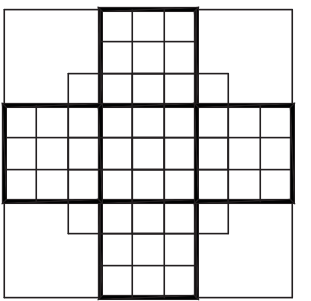
- EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 5th EDITION (2014) BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.
- WIND DESIGN PRESSURES NOTED MAY BE MULTIPLIED BY (6) FOR COMPARISON TO ALLOWABLE (NOMINAL) WIND PRESSURES OF TESTED ASSEMBLIES. PER 2014 FBC.
- REFER TO STRUCTURAL NOTES FOR ALL WIND LOAD PARAMETERS.
- CORNER DISTANCE, A = 3.5 FEET

ALLOWABLE (Wx0.6)						
COMPONENT & CLADDING WIND DESIGN PRESSURES						
PRESSURES BASED ON V _w	ROOF WIND LOADS					WALL WIND LOADS
	ROOF AREA (10 SF)					WALL AREA (10 SF)
Kd IS INCLUDED	1	2	3	2 (OH)	3 (OH)	4
PRESSURE (PSF)	26.43	26.43	26.43	19.44	19.44	45.87
SUCTION (PSF)	-41.98	-73.08	-73.08	-85.52	-143.83	-49.76

- EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 5th EDITION (2014) BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.
- REFER TO STRUCTURAL NOTES FOR ALL WIND LOAD PARAMETERS.
- CORNER DISTANCE, A = 3.5 FEET



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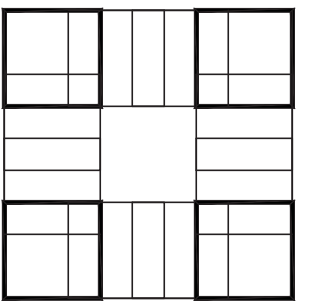


P. B. C.
R.G. KREUSLER PARK
RESTROOM BUILDING
AND ENTRY DRIVE
MODIFICATIONS

PBC NO 14204
2686 SOUTH OCEAN BLVD.

LAKE WORTH
FLORIDA

PROJECT NO.
201402



SHEET TITLE:
**WIND PRESSURES
STRUCTURAL NOTES
SCHEDULES**

REVISIONS:

DATE
11/20/15
DRAWN BY:
DG
CHECKED BY:
EP

SHEET
NUMBER

S-3



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