

**NOTES:**

- NO CHANGES SHALL BE MADE WITHOUT THE PRIOR PERMISSION OF THE ARCHITECT.
- THE CONTRACTOR SHALL CHECK ALL DIMENSIONS AND CONDITIONS OF THE JOB SITE PRIOR TO INITIATING CONSTRUCTION AND REPORT ANY DISCREPANCIES OR CONDITIONS THAT ARE UNSATISFACTORY.
- THE CONTRACTOR SHALL PERFORM ALL WORK IN STRICT COMPLIANCE WITH THE PLANS, SOUTH FLORIDA BUILDING CODE AND OTHER APPLICABLE CODES.
- CONTRACTOR SHALL NOT SCALE DRAWINGS. INFORMATION SHALL BE OBTAINED FROM ARCHITECT.
- SHOP DRAWINGS BEARING THE SEAL OF A FLORIDA REGISTERED ENGINEER SHALL BE SUBMITTED FOR APPROVAL OF ALL PREFABRICATED STRUCTURAL SYSTEMS.
- ALL WOOD IN DIRECT CONTACT WITH CONCRETE SHALL BE PROPERLY PRESURE TREATED WITH A WOOD PRESERVATIVE.
- AIR CONDITIONING (A/C) CONTRACTOR TO PROVIDE COMPLETE SHOP DRAWINGS OF DESIGN AND LAY-OUT FOR REVIEW.
- AIR HANDLER UNIT (AHU) TO HAVE BUILT-IN FACTORY DISCONNECTS.

**CODES AND STANDARDS**

- WIND LOADS WAS CALCULATED IN ACCORDANCE WITH ASCE 7-10.
- THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE 2014 FLORIDA BUILDING CODE
- BUILDING CODE REQUIREMENT FOR REINFORCED CONCRETE, ACI 318-II, MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES ACI 318-11.
- SPECIFICATION FOR THE DESIGN, FABRICATION & ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, ALSO ASD 360-05.
- BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES, ACI 530-II.

**FOUNDATION DESIGN**

- ALLOWABLE SOIL BEARING CAPACITY OF 2500 PSF WAS USED FOR DESIGN OF FOOTINGS. VERIFICATION SHALL BE PROVIDED BY GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION.
- SET TOP OF ALL FOOTINGS AT -1'-4" (UNO).
- FOOTING SHALL BE EXCAVATED TO CLEAN SOIL, FREE OF VEGETATION AND DELETERIOUS MATTER, AND CONCRETE SHALL BE PLACED ON AN UNDISTURBED BASE. (SEE SITE PREPARATION NOTE)

**CONCRETE**

CONCRETE TO REACH DESIRED STRENGTH AS NOTED ON PLANS AND SCHEDULES IN 28 DAYS (MINIMUM CONCRETE STRENGTH SHALL BE FC + 5000 PSI IN 28 DAYS) AND HAVE A MINIMUM OF 517 LBS. OF CEMENT PER CUBIC YARD. ALL CONCRETE SHALL BE "READY MIXED" AND IN ACCORDANCE WITH ASTM SPECIFICATIONS C-9. A CERTIFICATE OF MANUFACTURERS MIX AND STRENGTH NO WATER TO BE ADDED. ALL CONCRETE SHALL LEAVE PLANT WITHOUT APPROVAL OF ENGINEER OR PLANT ENGINEER. PLANT CONTROL IS REQUIRED. MAXIMUM MIX TIME AT POINT OF DEPOSIT IS 90 MINUTES.

**REINFORCING**

REINFORCING STEEL SHALL BE DEFORMED, NEW BILLET STEEL ASTM A-615 GRADE 60. SPLICES TO BE IN ACCORDANCE WITH ACI 318-II FOR "STRENGTH DESIGN". FABRICATION AND PLACING OF STEEL SHALL BE IN ACCORDANCE WITH 2002 ACI CODE STANDARDS AND PRACTICE PROCEDURES.

**REINFORCED MASONRY:**

- HOLLOW MASONRY UNITS SHALL CONFORM TO ASTM C-90, TYPE II, GRADE N, SQUARE END, WITH A MINIMUM AVERAGE CONCRETE STRENGTH ON OF FM + 1500 P.S.I.
- A) MORTAR SHALL CONFORM TO ASTM C-270, TYPE "M" WITH A 28-DAY STRENGTH OF 2500 P.S.I.  
B) FILL CELLS GROUT SHALL HAVE A MINIMUM STRENGTH OF 4000 PSI. W/ HIGH W/C RATIO AND A MINIMUM OF 8" SLUMP. GROUT SHALL CONFORM TO ASTM C-476.
- REINFORCED MASONRY WALLS ARE DESIGNED PER ACI 530-II.
- SPECIAL INSPECTION IS REQUIRED FOR THE REINFORCED MASONRY WALL.
- LAY ALL MASONRY WITH FULL FACE HEAD JOINTS AND WITH FACE SHELL MORTAR BEDDING.
- MASONRY ANCHORAGE TO SUBSTRUCTURE SHALL BE PROVIDED IN ACCORDANCE W/ STRUCTURAL DRAWINGS AND DETAILS.
- THE USE OF ADMIXTURES SHALL NOT BE PERMITTED WITHOUT THE PRIOR APPROVAL OF ENGINEER.
- VERTICAL REINFORCING:  
A) ASTM A 615-60 PER REINFORCING SECTION.  
B) WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL FOR ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.  
C) VERTICAL REINFORCING STEEL SHALL HAVE A MINIMUM CLEARANCE OF ONE-QUARTER INCH FROM THE MASONRY.  
D) VERTICAL REINFORCEMENT SHALL BE PROVIDED AT EACH SIDE OF OPENINGS IN WALL. AT WALL INTERSECTIONS, CORNERS AND ENDS, THIS REINFORCING SHALL BE THE SAME SIZE AS THE CORNER WALL REINFORCING FOR THE PARTICULAR WALL BUT NEVER LESS THAN #5 - UNLESS NOTED OTHERWISE. SPECIAL CARE SHALL BE TAKEN TO INSURE THAT CELLS TO BE GROUTED LINE UP PROPERLY.
- HORIZONTAL REINFORCING NO. 9 LADDER TYPE AT 16" O.C. VERTICALLY. MIN LAP OF 6".
- PROVIDE "DOVE TAIL" ANCHORS AT 16" O.C. VERTICAL FOR ALL MASONRY PLACED ADJACENT TO COLUMNS OR EXTEND HORIZONTAL REINFORCING 4" INTO COLUMN.
- HOOK ALL VERTICAL REINFORCING INTO BEAM ABOVE WHERE REBAR TERMINATES.
- PROVIDE CORNER BARS AT ALL CONCRETE/ROOF BEAM INTERSECTIONS. BARS SHALL BE SAME SIZE AND QUANTITY AS BEAM REINFORCING, EXTENDING 48 DB IN BOTH DIRECTIONS.
- IF ONE END OF A WALL IS HIGHER THAN THE INTERSECTED WALL, THE CORNER BARS SHALL BE TURNED DOWN INTO THE VERTICAL FILLED CELL.
- REFER TO FLOOR PLANS FOR VERTICAL STEEL SIZE AND SPACING IN THE REINFORCED MASONRY WALLS.
- SPLICE SHALL BE A MINIMUM OF 48 BAR DIAMETER FOR #5 & 52 BAR DIA. FOR #6.

**SLAB ON FILL:**

INTER CONCRETE SLABS POURED ON FILL TO BE POURED OVER A WATERPROOF MEMBRANE OVER CLEAN NON ORGANIC FILL. ALL SLABS BE REINFORCED WITH #3" W/4XW/4 WELDED WIRE FABRIC PLACED AT MID DEPTH OF SLAB, UNLESS NOTED OTHERWISE. MINIMUM SLAB THICKNESS EQUALS 4" UNLESS NOTED OTHERWISE.

**STRUCTURAL STEEL:**

- STRUCTURAL STEEL CONSTRUCTION SHALL CONFORM WITH THE REQUIREMENTS OF THE AISC'S SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL OR BUILDINGS, LATEST EDITION.
- STRUCTURAL STEEL PLACEMENT DRAWINGS AND MATERIAL LISTS SHALL CONFORM TO AISC'S 'STRUCTURAL STEEL DETAILING', LATEST EDITION. SHOP DRAWINGS SHALL BE PREPARED UNDER THE SUPERVISION OF A REGISTERED ENGINEER AND SUBMITTED FOR REVIEW PRIOR TO FABRICATION. (ONE PAPER SET/PA AND ONE PRINT OF EACH DRAWING)
- ALL STRUCTURAL MEMBERS AND MISC. METALS SHALL CONFORM WITH ASTM A-36 UNLESS NOTED OTHERWISE.
- SHOP CONNECTIONS SHALL BE WELDED IN ACCORDANCE WITH AWS D11, 'STRUCTURAL WELDING CODE'. USE E70XX ELECTRODE.
- ALL FILLED BOLTS SHALL BE ASTM A-325N 3/4" - BOLTS.
- STEEL TUBING TO BE ASTM - A500 GRADE B(46).
- ALL STEEL TO HAVE A SHOP COAT OF RUST INHIBITIVE PAINT.
- DELETE PAINT ON ALL STEEL TO RECEIVE SPRAYED ON FIREPROOFING OR CONCRETE ENCASEMENT BARS.
- FULL DEPTH CONNECTIONS ARE TO BE USED ON ALL GIRDER AND BEAM CONNECTIONS TO COLUMNS. BOLTS TO BE AT 3" O.C. VERT. (UNO).
- MINIMUM BEARING OF STEEL BEAMS AND LINTELS ON MASONRY SHALL BE 4".

**FORM WORK:**

SHORING AND BRACING PROCEDURES SHALL FOLLOW THE RECOMMENDED PRACTICE FOR CONCRETE FORM WORKING BASED UPON AMERICAN CONCRETE INSTITUTE STANDARD, ACI 347-04, ARTICLES 2.4 THROUGH 2.7. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL BRACING OF VERTICAL MEMBERS (WALL BEAMS AND COLUMNS) DURING CONSTRUCTION PHASE UNTIL FLOOR AND ROOF SYSTEM ARE IN PLACE AND PROPERLY ATTACHED TO SUCH MEMBERS. DATUM

RELATIVE ELEVATION TOP OF FLOOR SLAB TAKEN TO 0'-0". (SEE PLANS AND SCHEDULES) VERIFY ALL SLAB RECESSES WITH ARCHITECTURAL DRAWINGS.

**CONCRETE COVERAGE**

PER ACI 318, THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: \_\_\_\_\_ 3"
- CONCRETE EXPOSED TO EARTH OR WEATHER: \_\_\_\_\_ 2"  
NO. 6 THROUGH NO. 18 BARS: \_\_\_\_\_ 1-1/2"  
NO. 5 BAR AND SMALLER: \_\_\_\_\_
- CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:  
SLABS, WALLS, JOINTS: NO. 11 BAR AND SMALLER: \_\_\_\_\_ 3/4"  
BEAMS, COLUMNS: PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS: \_\_\_\_\_ 1-1/2"  
SHELLS, FOLDED PLATE MEMBERS: NO. 11 BAR AND SMALLER: \_\_\_\_\_ 3/4"

**MECHANICAL EQUIPMENT:**

LOCATION FOR ALL OPENINGS, MECHANICAL TRENCHES, ENCASED CONDUIT OR PIPING AND ANCHOR BOLTS SHALL BE VERIFIED WITH MANUFACTURER'S SHOP DRAWINGS PRIOR TO CONSTRUCTION. NOTIFY ARCHITECT OF ANY DISCREPANCIES.

**SHOP DRAWINGS:**

- SUBMIT (1) REPRODUCIBLE AND (2) COPIES OF EACH REQUIRED SUBMITTAL.  
(1) COPY OF WHICH WILL BE KEPT IN THIS OFFICE.
- ALLOW 5 WORKING DAYS FOR REVIEW TIME.

**CONCRETE CYLINDER AND SLUMP TESTS:**

AT LEAST ONE SET OF CYLINDERS SHALL BE PROVIDED FOR STRENGTH AND SLUMP TESTS PER POUR OR FOR EACH 50 CUBIC YARDS OF CONCRETE, WHICHEVER IS LESS. AT LEAST TWO SETS OF TESTS ARE RECOMMENDED FOR COLUMN POURS. FOR EACH POUR THE ENGINEER SHALL BE PROVIDED WITH ONE (1) 3-DAY TEST, ONE (1) 7-DAY TEST, ONE (1) 28-DAY TEST, ONE (1) SPARE, AND ONE (1) SLUMP TEST.

**FILL COMPACTION:**

SOIL SHALL BE COMPACTED TO 95 % MODIFIED PROCTOR (ASTM D1557-78) WITHIN A DISTANCE OF FIVE FEET BEYOND ALL BUILDING EDGES. AT LEAST ONE FIELD DENSITY TEST SHALL BE PERFORMED FOR EACH 1600 SQUARE FEET OF AREA. DENSITY TESTS ARE TO BE MADE 12" BELOW THE COMPACTED SURFACE. RESULTS OF PROCTOR TEST(S) AND FIELD DENSITY TEST(S) SHALL BE FURNISHED TO THE ENGINEER.

**STEEL JOIST GIRDER DESIGN:**

- CHORD AND WEB MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH STANDARD SPECIFICATIONS FOR JOIST GIRDERS OF THE STEEL JOIST INSTITUTE WITH THE FOLLOWING ADDITIONAL REQUIREMENTS.
- TOP CHORD - THE TOP CHORD SHALL BE DESIGNED AS A COMPRESSION MEMBER WITH PARTIAL LATERAL RESTRAINT. THE UNBRACED LENGTH IN THE LATER DIRECTION SHALL BE TAKEN AS THE SPACING BETWEEN THE JOISTS BEARING ON THE GIRDER.
- TENSION WEBS - IN ADDITION TO ALL OTHER DESIGN REQUIREMENTS OF THE STANDARD SPECIFICATIONS OF GIRDER JOIST, TENSION WEBS OF JOIST GIRDERS SHALL BE DESIGNED FOR A STRESS REVERSALS (COMPRESSION) EQUIVALENT TO A VERTICAL SHEAR OF NOT LESS THAN 25% OF THE END REACTION.
- COMPRESSION WEBS - COMPRESSION WEBS SHALL BE DESIGNED USING A STIFFNESS FACTOR OF 4.
- BEARING - JOIST GIRDERS SHALL BEAR AT LEAST 4" OVER STEEL ANCHOR SEATS (SEE DETAIL SHEET) AND FULLY WELDED TO SUPPORT.
- BRIDGING - JOIST GIRDERS SHALL BE PROPORTIONED SUCH THAT THEY CAN BE ERECTED WITHOUT BRIDGING. BOTTOM CHORD "X" BRACES SHALL BE FINISHED TO LIMITS THE LAY OF THE BOTTOM CHORD TO 240.
- STABILIZER PLATES - BOTTOM CHORD SHALL BE STABILIZED Laterally AT EACH END OF JOIST GIRDERS AS DETAILED ON PLANS. BOTTOM CHORDS SHALL NOT BE WELDED TO STABILIZER PLATES UNTIL ALL DEAD LOADS ARE PRESENT ON THE GIRDER JOIST.

**OPEN WEB STEEL JOIST:**

- ALL JOISTS SHALL HAVE A SHOP COAT OF RUST INHIBITIVE NON BITUMINOUS PAINT.
- JOIST FABRICATOR SHALL HAVE AN ENGINEER REGISTERED IN THE STATE OF FLORIDA CERTIFY IN WRITING THAT THE STEEL JOISTS CAN SAFELY RESIST THE WIND UPLIFT FORCES AS SHOWN ON PLANS, AS PER THE ANSI/ASCE 7-02.
- STEEL JOISTS SHALL BE DESIGNED, FABRICATED, AND ERECTED TO THE REQUIREMENTS OF THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE. MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE. PROVIDE BRIDGING IN ACCORDANCE WITH SJI STANDARDS (UNO).
- MINIMUM BEARING OWSJ SHALL BE:  
K SERIES ON MASONRY ON CONCRETE ON STEEL  
4" 4" 2 1/2"  
LH SERIES 6" 6" 4"
- CAMBER JOISTS FOR DEAD LOAD.
- WELD OR BOLT EVERY OWSJ TO SUPPORTING STEEL AND/ OR WALL PLATES.
- MAXIMUM DEFLECTION OF JOISTS AND ROOF DECK 1/360 OF SPAN UNDER LIVE LOAD.
- FABRICATOR SHALL ADD WEIGHTS OF ALL MECHANICAL ROOF UNITS IN THE DESIGN LOAD OF OWSJ AND VERIFY WITH THE OWNER OR MECHANICAL ENGINEER.
- FABRICATOR SHALL SUBMIT DESIGN DATA AND ERECTION DIAGRAMS. SHOP DRAWINGS SHALL BE PREPARED UNDER THE SUPERVISION OF A REGISTERED PROFESSIONAL ENGINEER.
- FRAME ROOF OPENING LARGER THAN 12'X12' WITH C5X6.7 SADDLES UNLESS NOTED OTHERWISE.
- FRAME ROOF TOP AC OPENINGS AS PER DETAIL SHOWN ON THIS SHEET.

**ROOF DECK:**

- STEEL ROOF DECK SHALL BE 1/2" (22 1/2 GA. (UNO)) TYPE B METAL DECK GALVANIZED AS SHOWN ON ROOF PLAN AND AS MANUFACTURED BY VULCRAFT OR APPROVED EQUAL. MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE.
- ROOF DECK SHALL CONFORM WITH STEEL DECK INSTITUTE STANDARDS.
- STEEL ROOF DECK SHALL BE FASTENED TO PERFORM AS A HORIZONTAL DIAPHRAGM CAPABLE OF RESISTING LOAD SPECIFIED ON PLAN.
- STEEL ROOF DECK SHALL BE FASTENED TO SUPPORT AS PER NOTES ON ROOF PLAN.

**MISC. STEEL:**

CONTRACTOR SHALL INCLUDE IN HIS BID THE COST OF 1,000 POUNDS OF ADDITIONAL STRUCTURAL STEEL INCLUDING FABRICATION, FURNISHING, AND INSTALLING AS REQUIRED FOR USES DIRECTED BY THE ARCHITECTS, THE ARCHITECT'S AGENT OR BY THE OWNER'S REPRESENTATIVE.

**CAST IN PLACE LINTELS:**

SPANS UP TO 6'-4" - 8'X12" WITH 4-#5 AND TIES AT 4" (MIN) EXTEND LINTEL 8" BEYOND OPENING. (MIN) PROVIDE DOWELS FROM COLUMNS TO MATCH LINTEL STEEL WHEN FRAMING INTO COLUMNS. PROVIDE 1" X 8" SHEAR KEY AT COLUMN FACE. LAP STEEL 5" X BAR NUMBER.

**PRECAST-PRESTRESSED CONCRETE LINTELS:**

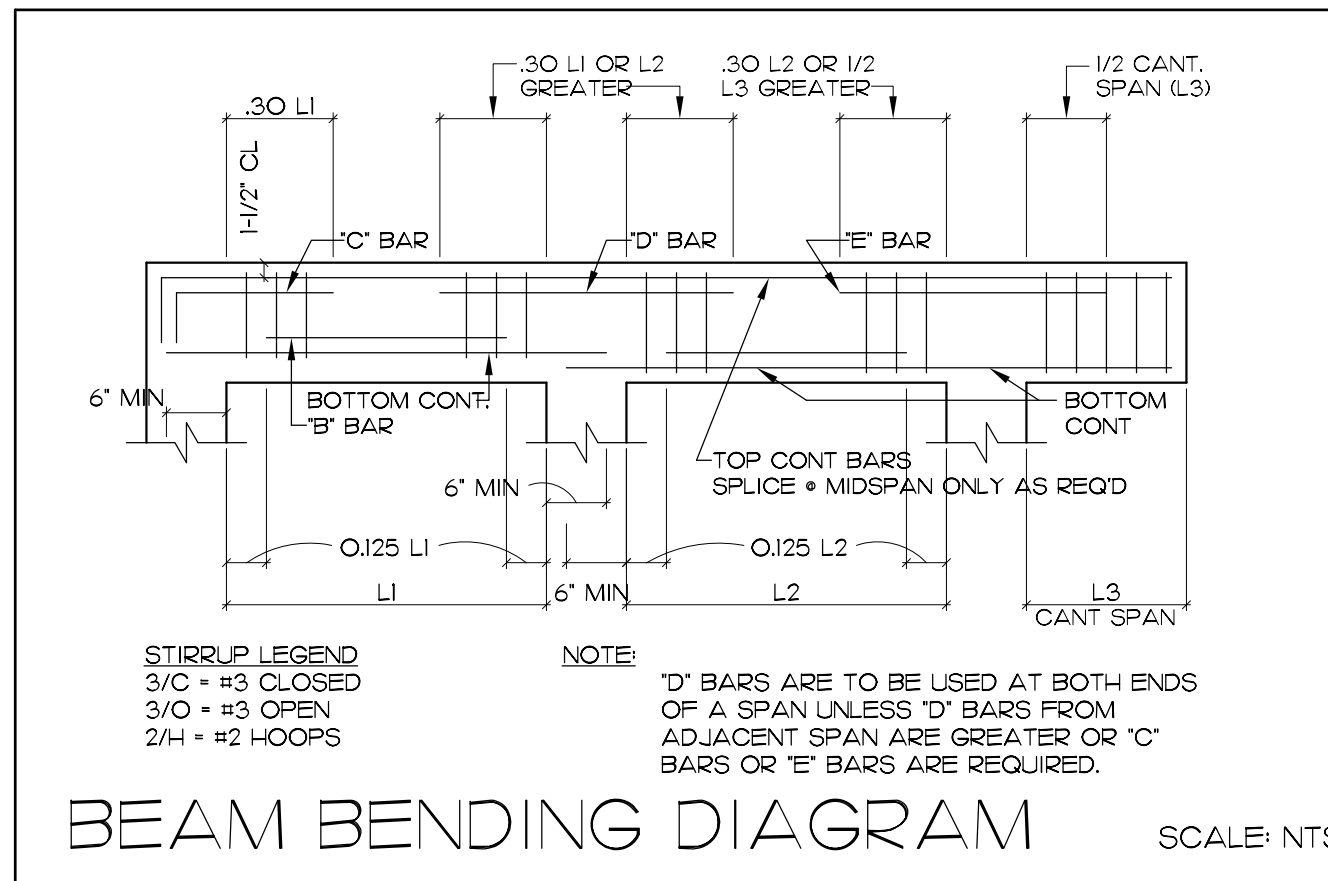
- CONCRETE FOR PRECAST-PRESTRESSED UNITS SHALL BE DESIGNED TO ATTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 5000 PSI CONCRETE.
- PRESTRESSING STRANDS SHALL BE EQUIVALENT TO LATEST ASTM A416-59T.
- MANUFACTURER TO SUBMIT SHOP DRAWINGS TO ARCHITECT PRIOR TO FABRICATION.
- THE PRE STRESSED CONCRETE SHOP DRAWINGS SUBMITTED BY MANUFACTURER, WHICH SHALL INCLUDE COMPLETE DESIGN CALCULATIONS, SHALL BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.
- ALL PRE STRESSED CONCRETE MEMBERS SHALL MAINTAIN A POSITIVE UPWARD CAMBER UNDER FULL DEAD LOAD. MEMBERS WITH ZERO OR NEGATIVE CAMBER WILL NOT BE ACCEPTABLE. CAMBER DATA SHALL BE INCLUDED IN CALCULATIONS SUBMITTED BY MANUFACTURER AND VERIFIED IN THE FIELD.

**CONTROL JOINTS:**

CONTRACTOR SHALL SAW CUT CONTROL JOINTS IN CONCRETE SLAB WITHIN 24 HOURS OF PLACING CONCRETE. CONTROL JOINTS SHALL BE LAID OUT AT CENTER LINES OF COLUMNS WHERE POSSIBLE, AND SHALL NOT EXCEED 18'-0" X 18'-0" SQUARE PANELS. LONGER DIMENSION OF PANEL SHALL NOT EXCEED 1.5 TIMES THE SHORTER ONE. SAWCUT SHALL BE 1/4 OF THE SLAB DEPTH AND 1/8" WIDE.

**COLUMN TIES:**

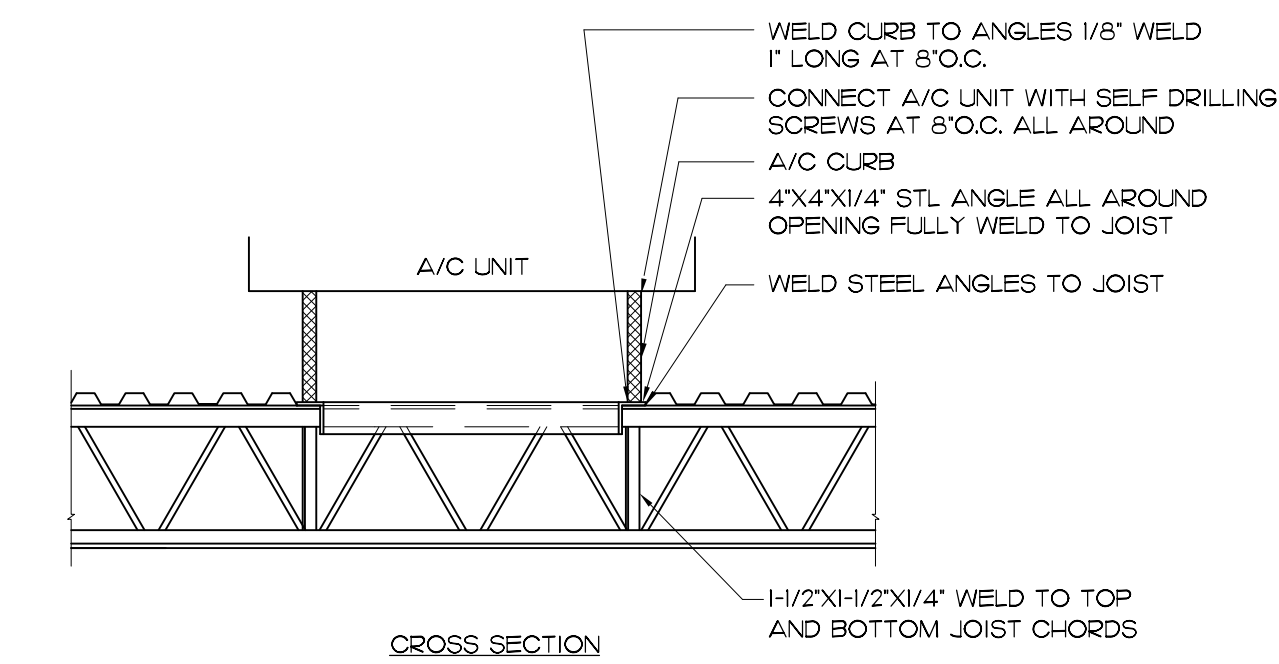
- COLUMN TIE SPACING SHALL BE EQUAL TO THE LEAST DIMENSION OF THE COLUMN UNLESS NOTED OTHERWISE.
- TIES SHALL BE 1/2" VERTICAL BARS #10 AND SMALLER, AND #4 FOR VERTICAL BARS LARGER THAN #10.
- TIE SPACING SHALL BEGIN NOT MORE THAN HALF A TIE SPACING ABOVE THE FLOOR (FOUNDATION AT FIRST FLOOR) AND SHALL END NOT MORE THAN HALF A TIE SPACING BELOW THE BOTTOM SLAB REINFORCING ABOVE.
- TO BE 48 BAR DIAMETER FOR SPLICE LENGTHS.



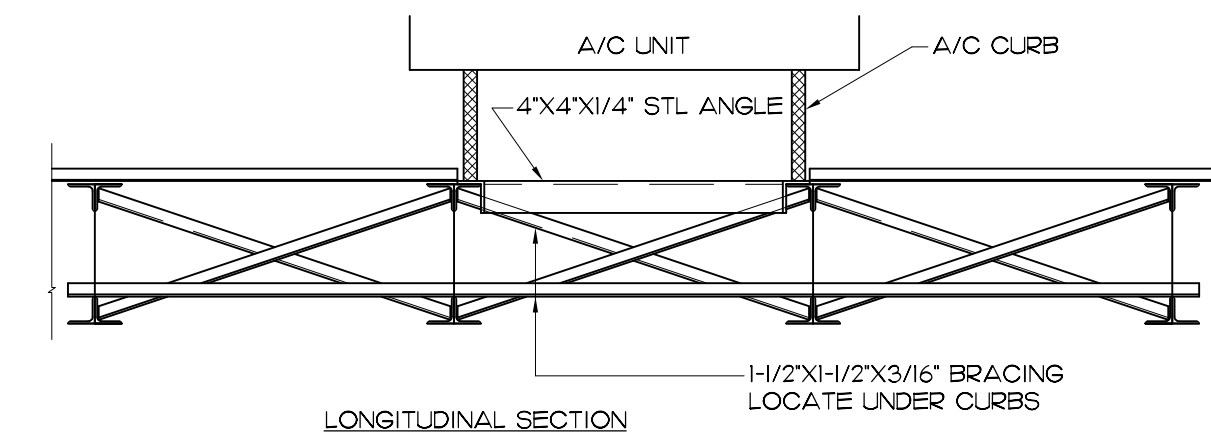
**BEAM BENDING DIAGRAM** SCALE: NTS

**GENERAL BEAM SCHEDULE NOTES:**

- SCHEDULED HOOPS OR STIRRUPS SHALL BE PLACED AT EACH END OF BEAM UNLESS OTHERWISE NOTED.
- BUNDLE ALL STRUCTURAL BEAM TOP BARS IN PAIRS OVER SUPPORT WITH TOP BARS FROM ADJACENT BEAMS.
- ALL CONCRETE BEAMS OTHER THAN THOSE WITH THE PREFIX TB SHALL BE POURED PRIOR TO PLACING BLOCK BELOW.
- ALL TIE BEAM REINFORCING SHALL EXTEND INTO SPAN OF ANY ADJACENT STRUCTURAL BEAM AS PER BEAM BENDING DIAGRAM.
- DROP BOTTOM OF THE BEAM AS REQUIRED AT WINDOW AND DOOR HEADS (28' MAXIMUM) AND ADD 2 #5 BOTTOM IF DROP EXCEEDS 8'.
- THE BEAM SCHEDULED DEPTHS ARE MINIMUM AND MAY BE INCREASED (6" MAXIMUM) TO FIT BLOCK WORK.
- ALL ADDED LONGITUDINAL BEAM REINFORCING SHALL EXTEND 6" MINIMUM INTO SUPPORT AND BEAMS SHALL HAVE 8" MIN. BEARING.
- PROVIDE CORNER BARS AT ALL CONCRETE TIE BEAM INTERSECTIONS. BARS SHALL BE SAME SIZE AND QUANTITY AS BEAM REINFORCING, EXTENDING 48 BAR DIAMETER.



**CROSS SECTION**

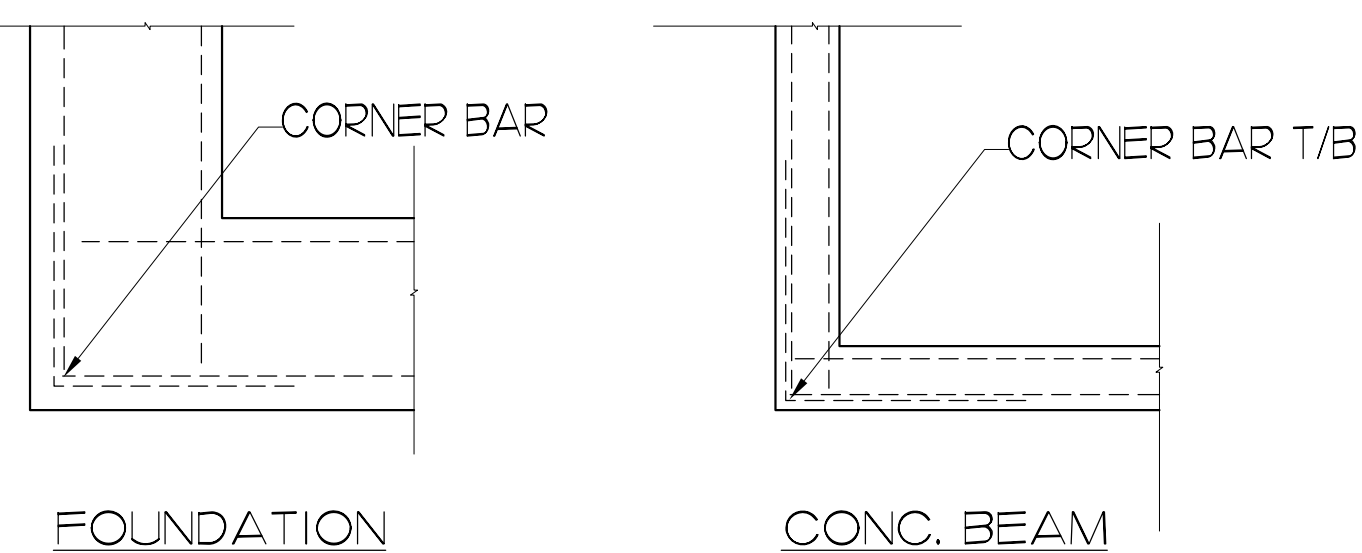


**LONGITUDINAL SECTION**

**A/C UNIT SUPPORT DETAIL NOTES:**

- WELD STEEL ANGLES TO JOISTS.
- WELD CURB TO ANGLES WITH 1/8" WELD X 1' LONG AT 8" O.C.
- CONNECT A/C UNIT TO CURB WITH SELF DRILLING SCREWS AT 8" O.C. ALL AROUND.
- IF UNIT SIZE DOES NOT MATCH JOIST SPACING, STEEL SUPPORT ANGLE SHALL SPAN BETWEEN JOISTS WITH BRIDGING AS SHOWN IN SPACE TO EITHER SIDE OF END OF ANGLE AS WELL AS UNDERNEATH AS SHOWN.

A/C UNITS MUST BE SUPPORTED OVER 2 JOISTS (TYPICAL)



**FOUNDATION**

**CONC. BEAM**

**CORNER BARS**

- CORNER BARS SHALL BE PLACED AT OUTSIDE FACE OF ALL BEAMS & FTG'S
- CONCRETE BEAMS SHALL HAVE CORNER BARS AT TOP & BOTTOM
- CORNER BARS SHALL BE THE SAME SIZE AS THE BEAM REINFORCING AND SHALL HAVE A 48 DB SPLICE IN EACH DIRECTION (30" FOR #5, 36" FOR #6, ETC.)
- IF UNIT SIZE DOES NOT MATCH JOIST SPACING, STEEL SUPPORT ANGLE SHALL SPAN BETWEEN JOISTS WITH BRIDGING AS SHOWN IN SPACE TO EITHER SIDE OF END OF ANGLE AS WELL AS UNDERNEATH AS SHOWN.



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SECOND FLOOR ADDITION FOR  
**DIXIE DIVERS**

455 U.S. HIGHWAY #1  
DEERFIELD BEACH, FL.

drawing name:

revisions

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drawn	H. DeMello
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