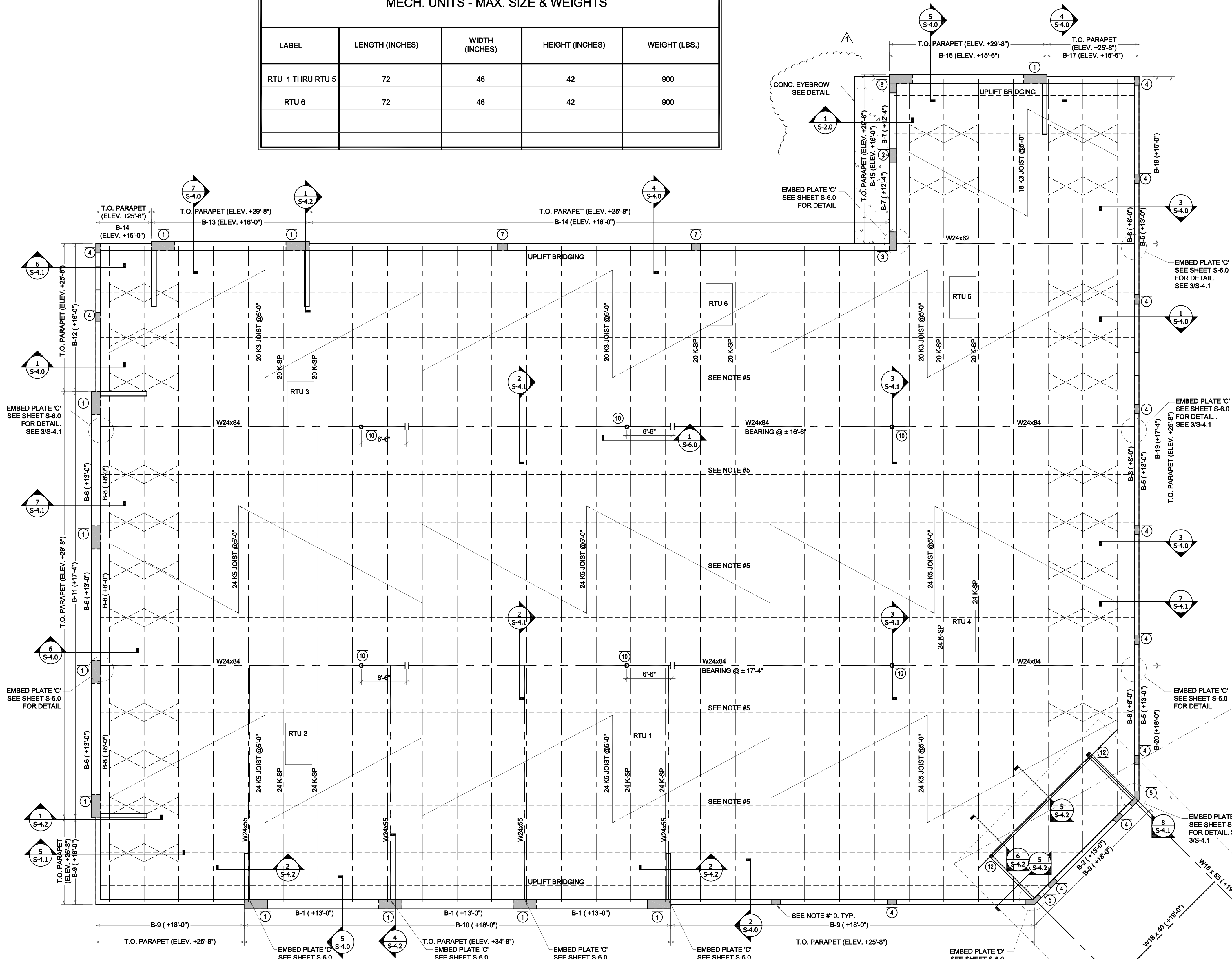


| MECH. UNITS - MAX. SIZE & WEIGHTS | | | | |
|-----------------------------------|-----------------|----------------|-----------------|---------------|
| LABEL | LENGTH (INCHES) | WIDTH (INCHES) | HEIGHT (INCHES) | WEIGHT (LBS.) |
| RTU 1 THRU RTU 5 | 72 | 46 | 42 | 900 |
| RTU 6 | 72 | 46 | 42 | 900 |



ROOF FRAMING PLAN:

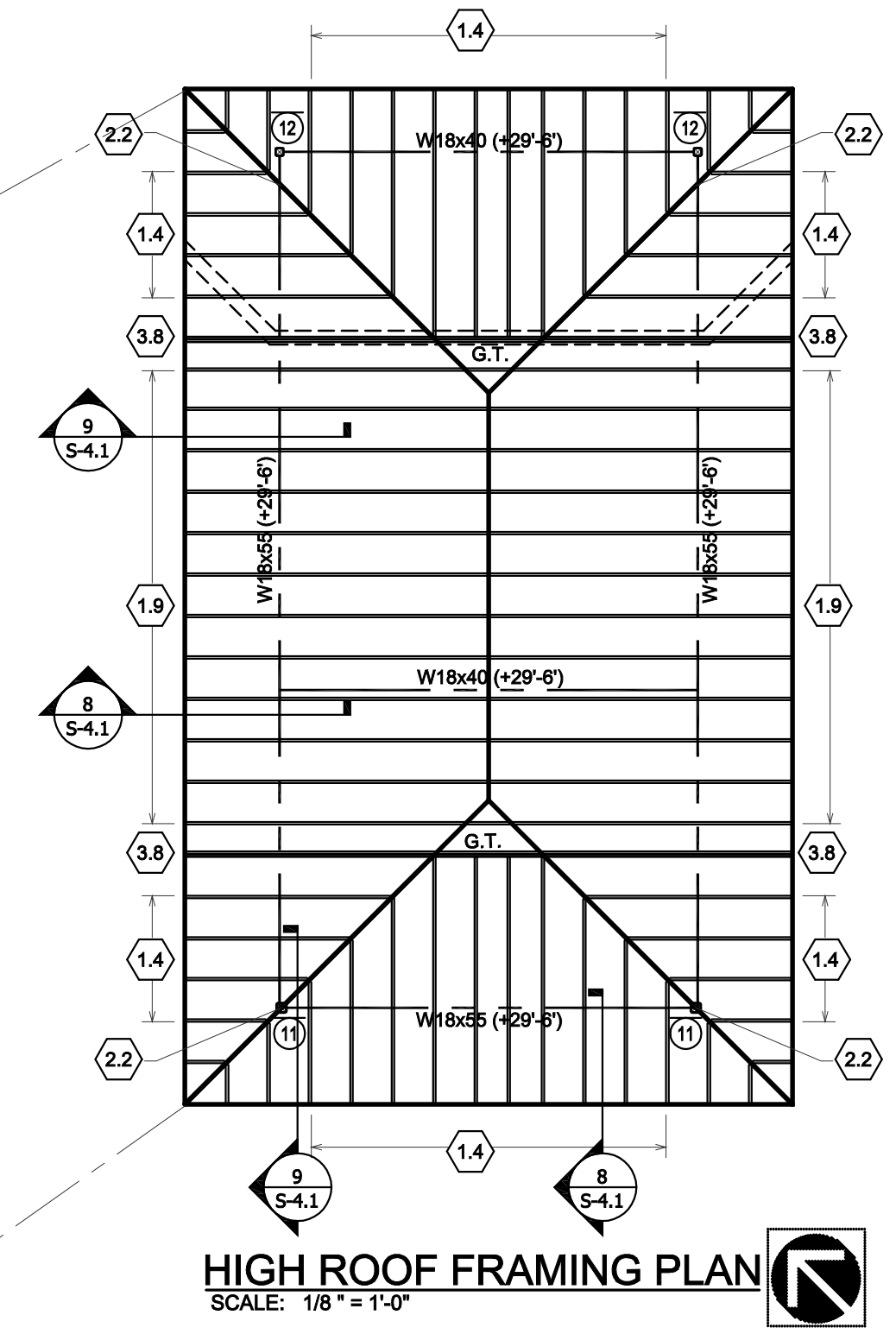
- ROOF STRUCTURE SHALL BE METAL DECK 1.5B - 18GA. GALVANIZED G90 TYPE "B" METAL DECK ON STEEL JOISTS WITH THE FOLLOWING DECK FASTENING PATTERN: 5/8"Ø PUDDLE WELDS AT ALL EDGE & PERIMETER OF DISCONTINUITIES @ 6"Ø.c. AND 3Ø7 - PATTERN AT SIDELAPS. SEE ROOF ATTACHMENT DIAGRAM FOR ADDITIONAL INFORMATION ON S-4.1. SEE ARCHITECTURAL PLAN FOR INSULATION SPECIFICATIONS ON SELECTION OF VENTED OR NON VENTED METAL DECK.
- PROVIDE CONTINUOUS L 4"x4"x1/4" AT ALL STEEL ROOF DECK PERIMETER, FASTENED WITH 5/8"Ø X 6 1/2" LONG SIMPSON TITEN HD SPACED @ 24"Ø.c. MAX.
- STEEL JOISTS DESIGN CRITERIA: LL = 30 lbs/sqft., DL = 25 lbs/sqft. WHERE "SP" IS DESIGNATED, JOIST DESIGNER SHALL INCLUDE THE INDICATED SUPERIMPOSED LOAD IN THE JOIST DESIGN.
 - RTU LOADS:
D + W = 210 PLF (DOWNWARD)
0.8Ø + W = 320 PLF (UPLIFT)
- ALL STEEL JOISTS SHALL ALSO BE DESIGNED FOR UPLIFT AND POSITIVE WIND LOADS. REDUCE BY 5 PSF GROSS UPLIFT SHOWN ON S-5.0 TO OBTAIN NET UPLIFT.
- ROWS OF PERMANENT JOIST BRIDGING SHALL BE INSTALLED FOR ALL JOISTS. BRIDGING SHALL BE 1 1/2" X 1 1/2" X 7/8" STEEL ANGLES. ADDITIONALLY, UPLIFT BRIDGING SHALL BE PROVIDED NEAR THE FIRST BOTTOM CHORD PANEL POINT AT EACH END OF EACH JOIST. THE SECOND AND THIRD SPACING FROM WALL / BEAM SHALL BE BRIDGING "X" TYPE. ALL BRIDGING SHALL BE FURNISHED AND INSTALLED TO MEET THE SIZE AND SPACING REQUIREMENTS OF THE SJI STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOIST. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOIST.
- COORDINATE ROOF TOP OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION.
- SEE ARCHITECTURAL DRAWINGS FOR LIGHT WEIGHT TOPPING SPECS IF REQUIRED.
- ALL STEEL JOIST SHALL BE DESIGNED FOR AN AXIAL SERVICE WIND LOAD, IN KIPS (1000 lbs = 1 KIP), OF 9 KIPS TO BE INCLUDED IN THE DESIGN BY THE ROOF JOIST MANUFACTURER.
- TOP OF CONC. COL = TOP OF PARAPET - TYP U.N.O.
- PROVIDE PRE CAST LINTEL OR REINFORCED CONCRETE LINTEL WHERE BEAMS ARE NOT DESIGNATED OVER OPENINGS. SEE REINFORCED CONCRETE LINTEL SCHEDULE ON SHEET S-6.0.
- SEE DETAIL 3/S5.0 FOR ADDITIONAL FRAME AT OPENINGS ON STEEL JOIST ROOF SYSTEM.
- SEE DETAIL 4/S-5.0 FOR RTU ATTACHMENT DETAIL.

ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

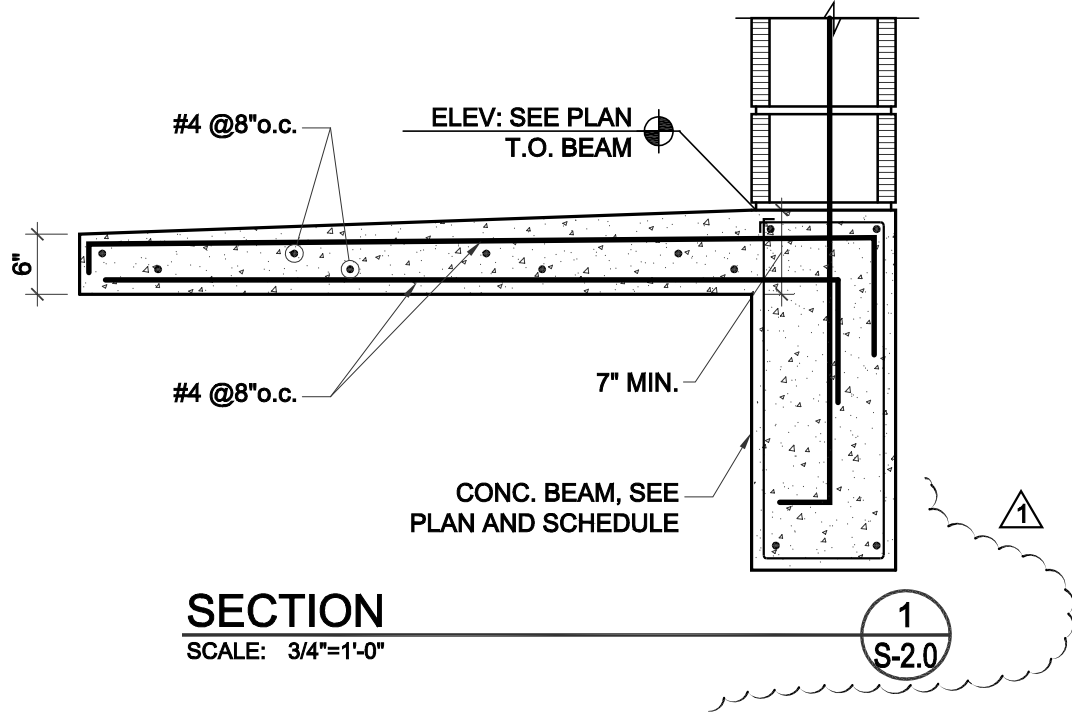


ROOF FRAMING PLAN NOTES:

- THE ROOF STRUCTURE IS 5/8" C.D.X. PLYWOOD ON PRE ENG. WOOD TRUSSES AT 24"Ø.c. PLYWOOD NAILING: 8d RING SHANK NAILS SPACED @ 4"Ø.c. AT ALL PERIMETER AND EDGES, AND AT 8"Ø.c. OVER INTERMEDIATE SUPPORTS.
- PRE ENG. WOOD TRUSSES SHALL BE DESIGNED FOR A SUPERIMPOSED LIVE LOAD OF 30 lbs/sqft AND A DEAD LOAD OF 25 lbs/sqft. AT ATTIC TRUSSES, DESIGN TRUSS BOTTOM CHORD FOR ADDITIONAL DEAD LOAD OF 10 lbs/sqft AND LIVE LOAD OF 20 lbs/sqft (UNINHABITABLE ATTIC w/ STORAGE). TRUSS DESIGN AND INSTALLATION SHALL COMPLY WITH THE FLORIDA BUILDING CODE. TRUSS DESIGNER SHALL SUBMIT SHOP DRAWINGS. TRUSS MANUFACTURER MUST SUPPLY ON TRUSS CALCULATIONS COMPONENTS AND CLADDING UPLIFT VALUES. INDICATE ALL REQUIRED CONNECTIONS, BRIDGING, CROSS BRACING AND LAY OUT AND SHALL BEAR THE SEAL OF A FLORIDA REGISTERED ENGINEER. ALL TRUSS TO TRUSS CONNECTIONS ARE THE RESPONSIBILITY OF THE TRUSS SPECIALTY ENGINEER.
- PROVIDE PERMANENT BRACING OF TRUSSES IN ACCORDANCE WITH THE REQUIREMENTS OF BRACING WOOD TRUSSES. COMMENTARY AND RECOMMENDATIONS PREPARED BY TRUSS PLATE INSTITUTE, INC.
- THE TRUSS FRAMING LAYOUT SHOWN IS SCHEMATIC IN NATURE, HOWEVER THE SUPPORTING STRUCTURE HAS BEEN DESIGNED FOR THE TRUSS ENGINEER TO CLOSELY MATCH FINAL TRUSS DESIGN, WITHOUT ALTERING LOAD PATHS. IF TRUSS ENGINEER ENCOUNTERS PROBLEMS WITH OUR ROOF TRUSS LAYOUT, PLEASE CONTACT UNISON STRUCTURAL DESIGN, LLC. IMMEDIATELY TO DISCUSS POSSIBLE SOLUTIONS.
- TRUSS MANUFACTURER TO COORDINATE VAULTED CEILINGS AND ATTIC SPACE WITH ARCHITECTURAL DWGS.
- (Ø) DESIGNATES UPLIFT IN KIPS (1.0k = 1,000 lbs). (U) = 1.0 kip OF UPLIFT. SEE UPLIFT TIE-DOWN SCHEDULE ON SHEET "S5.1" FOR APPROPRIATE TIE DOWNS.



HIGH ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"



SECTION
SCALE: 3/4" = 1'-0"

- Revisions:
- 1. COORDINATION 09/13/2016
 - 2.
 - 3.
 - 4.
 - 5.

Project Number
160412

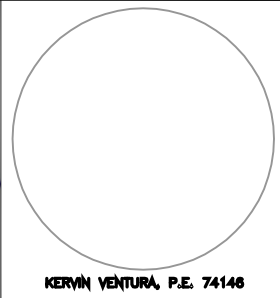
1066 U.S. HIGHWAY 1
New Retail Building - Goodwill
1066 U.S. HIGHWAY 1
Vero Beach, Florida

Project Name
New Retail Building

Stephen Brasgalla, Architect
 State Of Florida
 Registration No. AR12239
 6991 West Broward Boulevard Suite 100
 Plantation, Florida 33317
 Telephone 954.614.3801
 Telefax 954.208.0600
 architect@design23.net

Drawn By: **RSS** Checked By: **STB**
 Scale: **SHOWN** Date: **7-13-16**
 Project Number
160412

Sheet:
S-2.0



PERMIT SET 07-29-2016

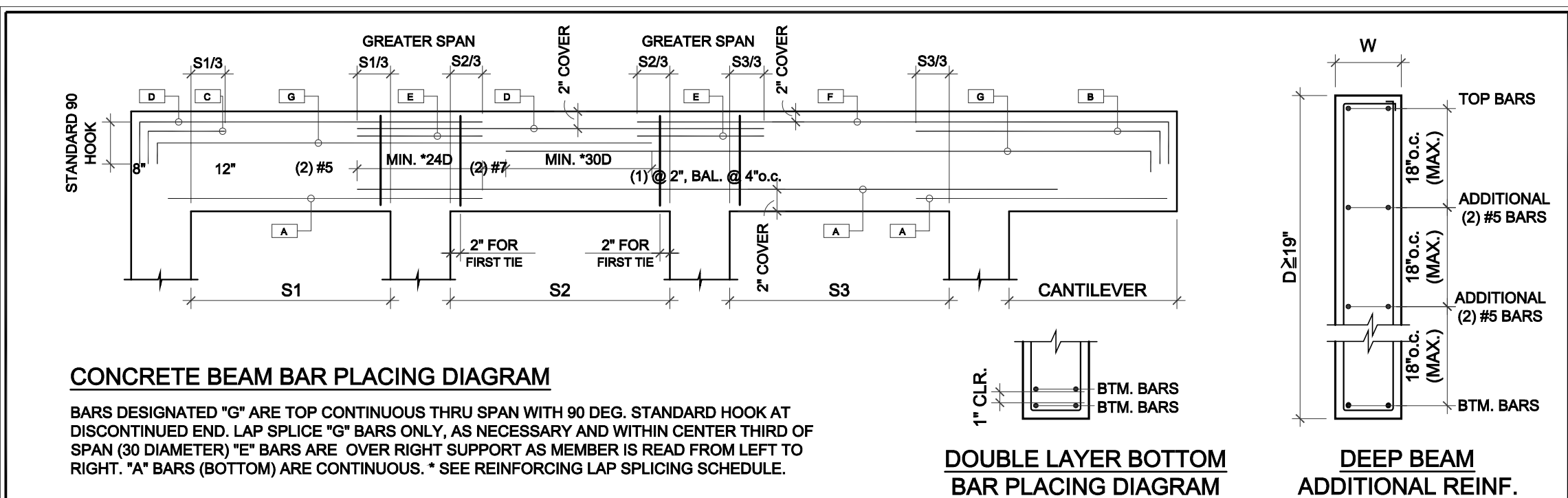
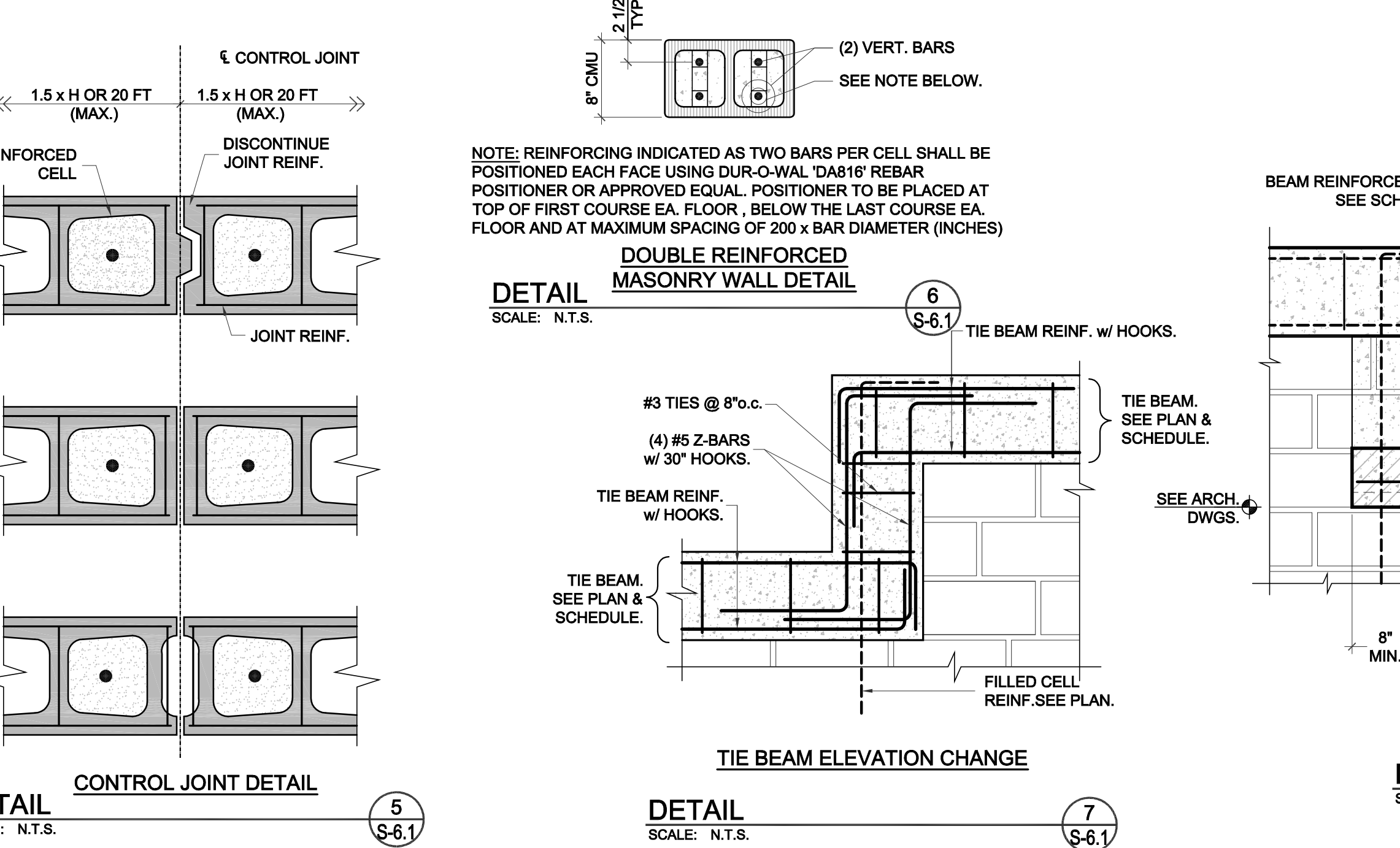
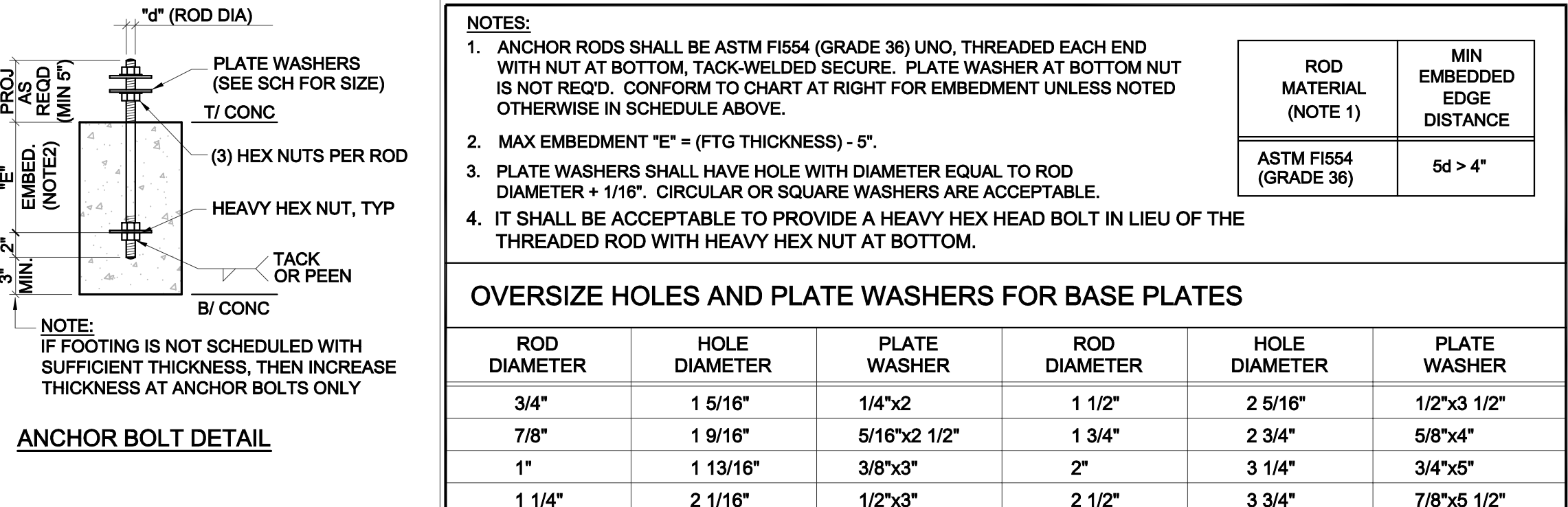
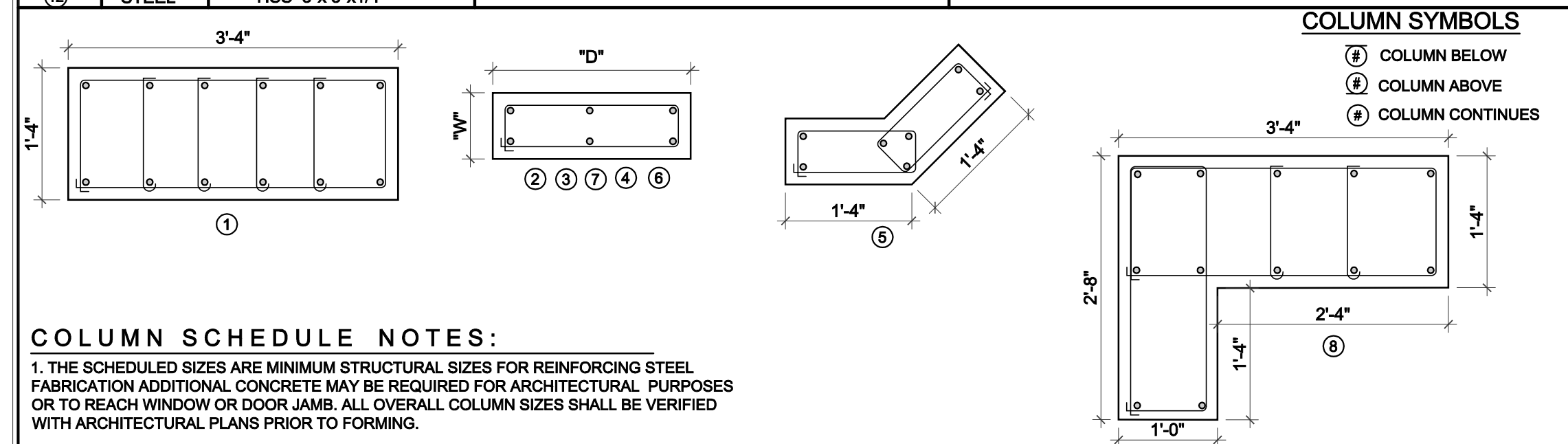
FOOTING CONCRETE STRENGTH, $f'_c = 3,000$ PSI

SOIL SAFE BEARING = 2500 PSF

| MARK | SIZE 'W' X 'D' | REINFORCING | REMARKS |
|-------|-----------------------|---|---------|
| TE-1 | 8" x 8" X CONT. | (1) #5 MID DEPTH. | - |
| F-36 | 3'-6" X 3'-6" X 12" | (5) #4 BOTTOM, EACH WAY. | - |
| F-46 | 4'-6" X 4'-6" X 14" | (5) #5 BOTTOM, EACH WAY. | - |
| F-50 | 5'-0" X 5'-0" X 14" | (5) #5 BOTTOM, EACH WAY. | - |
| F-56 | 5'-6" X 5'-6" X 16" | (7) #5 BOTTOM, EACH WAY. | - |
| F-66 | 6'-6" X 6'-6" X 18" | (6) #6 BOTTOM, EACH WAY. | - |
| F-76 | 7'-6" X 7'-6" X 20" | (8) #6 BOTTOM, EACH WAY. | - |
| F-110 | 11'-0" X 11'-0" X 28" | (10) #7 BOTTOM, (10) #5 TOP. | - |
| WF-16 | 16" X 12" X CONT. | (2) #5 CONT. BOTTOM, (1) #5 CONT., TOP. | - |
| WF-24 | 24" X 12" X CONT. | (3) #5 CONT. BOTTOM, (1) #5 CONT., TOP, PLUS #4 @ 12" o.c. TRANSVER., TOP & BOTT. | - |
| WF-48 | 48" X 12" X CONT. | (4) #5 CONT. BOTTOM, (2) #5 CONT., TOP, PLUS #4 @ 9" o.c. TRANSVER., TOP & BOTT. | - |

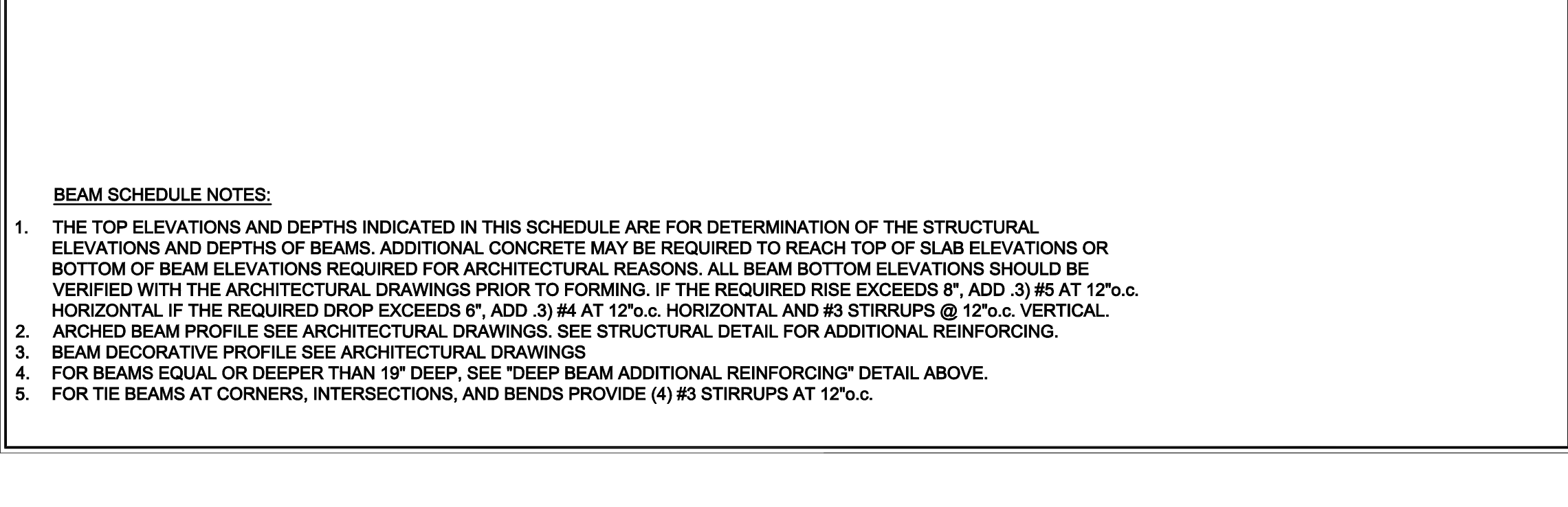
COLUMN SCHEDULE

| MARK | TYPE | SIZE 'W' X 'D' | VERTICAL REINFORCING | TIE SPACING | REMARKS |
|------|-------|----------------|--|---|---------|
| (1) | CONC. | 16 x 40 | (12) #8 | (1) #3 @ 8" o.c. | |
| (2) | CONC. | 12 x 24 | (6) #7 | (1) #3 @ 8" o.c. | |
| (3) | CONC. | 12 x 28 | (6) #7 | (1) #3 @ 8" o.c. | |
| (4) | CONC. | 8 x 16 | (6) #7 | (1) #3 @ 8" o.c. | |
| (5) | CONC. | 8 x 12 x 16 | (7) #7 | (1) #3 @ 8" o.c. | |
| (6) | CONC. | 8 x 20 | (6) #7 | (1) #3 @ 8" o.c. | |
| (7) | CONC. | 12 x 16 | (6) #7 | (1) #3 @ 8" o.c. | |
| (8) | CONC. | SEE DETAIL | (12) #8 | (1) #3 @ 10" o.c. | |
| (10) | STEEL | HSS 6"x6"x1/2" | BASE PLATE 18"x18"x1" w/ (8) 1"Ø THREAD BOLTS. | SEE ADD'L SECTIONS & DETAILS FOR BASE & CAP PLATE'S INFO. | |
| (11) | STEEL | HSS 8"x8"x1/2" | BASE PLATE 18"x18"x1" w/ (8) 1"Ø THREAD BOLTS. | SEE ADD'L SECTIONS & DETAILS FOR BASE & CAP PLATE'S INFO. | |
| (12) | STEEL | HSS 5"x5"x1/4" | | | |



BEAM SCHEDULE

| MARK | ELEVATION | NOTE-1 | BOTTOM REINFORCING | | TOP REINFORCING | | STIRRUPS | REMARKS |
|------|-----------------|--------|--------------------|-----------|-----------------|-----------|--|-----------------------------|
| | | | WIDTH 'B' | DEPTH 'D' | 'A' | 'C' & 'E' | | |
| PCL | SEE ARCH. DWGS. | 8 | 8 | (1) #5 | - | N/A | N/A | - |
| B-1 | +13'-0" | 16 | 36 | (3) #8 | - | (3) #6 | (5) @ 6" o.c., EA. END, BAL. @ 12" o.c. | - |
| B-2 | +13'-0" | 8 | 36 | (2) #7 | - | (2) #7 | (5) @ 6" o.c., EA. END, BAL. @ 12" o.c. | (2) #6 @ MID-DEPTH EA. FACE |
| B-3 | NOT USED | | | | | | | |
| B-4 | NOT USED | | | | | | | |
| B-5 | +13'-0" | 8 | 36 | (4) #8 | - | (2) #7 | (5) @ 4" o.c., EA. END, BAL. @ 8" o.c. | 2 LAYERS OF (2) #8 |
| B-6 | +13'-0" | 16 | 36 | (4) #8 | - | (4) #6 | (5) @ 4" o.c., EA. END, BAL. @ 8" o.c. | |
| B-7 | +12'-4" | 12 | 30 | (2) #7 | (2) #5 | (2) #5 | (4) @ 4" o.c., EA. END, BAL. @ 8" o.c. | |
| B-8 | +8'-0" | 8 | 16 | (2) #5 | - | (2) #5 | (5) @ 6" o.c., EA. END, BAL. @ 12" o.c. | (2) #5 @ MID-DEPTH EA. FACE |
| B-9 | +18'-0" | 8 | 20 | (2) #5 | (2) #5 | (2) #5 | (4) @ 12" o.c., EA. END, BAL. @ 24" o.c. | |
| B-10 | +18'-0" | 16 | 20 | (4) #6 | (2) #5 | (4) #6 | @ 12" o.c. | |
| B-11 | +17'-4" | 16 | 20 | (4) #6 | (2) #5 | (4) #6 | @ 12" o.c. | |
| B-12 | +16'-0" | 8 | 20 | (2) #5 | (2) #5 | (2) #5 | (4) @ 12" o.c., EA. END, BAL. @ 24" o.c. | |
| B-13 | +16'-0" | 16 | 24 | (4) #6 | (2) #5 | (4) #6 | @ 12" o.c. | |
| B-14 | +16'-0" | 12 | 16 | (2) #5 | (2) #5 | (2) #5 | (4) @ 12" o.c., EA. END, BAL. @ 24" o.c. | |
| B-15 | +16'-0" | 12 | 20 | (4) #6 | (2) #5 | (4) #6 | @ 12" o.c. | |
| B-16 | +15'-6" | 16 | 20 | (4) #6 | (2) #5 | (4) #6 | @ 12" o.c. | |
| B-17 | +15'-6" | 12 | 16 | (2) #5 | (2) #5 | (2) #5 | (4) @ 12" o.c., EA. END, BAL. @ 24" o.c. | |
| B-18 | +16'-0" | 8 | 20 | (2) #5 | (2) #5 | (2) #5 | (4) @ 12" o.c., EA. END, BAL. @ 24" o.c. | |
| B-19 | +17'-4" | 8 | 20 | (2) #5 | (2) #5 | (2) #5 | (4) @ 12" o.c., EA. END, BAL. @ 24" o.c. | |
| B-20 | +18'-0" | 8 | 20 | (2) #5 | (2) #5 | (2) #5 | (4) @ 12" o.c., EA. END, BAL. @ 24" o.c. | |



CAST IN PLACED REINFORCED CONCRETE LINTEL SCHEDULE

| WIDTH | DEPTH (MIN.) | CLEAR SPAN FT. | REINFORCING | | TIES |
|--------|--------------|-----------------|-------------|--------|--------------|
| | | | TOP | BOT | |
| 7 5/8" | 12" | 3'-0" TO 7'-0" | (2) #5 | (2) #5 | #3 @ 5" o.c. |
| 7 5/8" | 16" | 7'-0" TO 12'-0" | (2) #5 | (2) #5 | #3 @ 6" o.c. |

PROVIDE 8" MINIMUM BEARING AT EACH SIDE OF OPENING

REINFORCED CONCRETE LINTEL SCHEDULE

| WIDTH | HEIGHT | CLEAR SPAN IN FT. | REINFORCING | | TIES |
|---------|--------|-------------------|-------------|--------|---------------|
| | | | TOP | BOT | |
| 7-5/8" | 7-5/8" | UP TO 3'-0" | (2) #4 | (2) #4 | #2 @ 12" O.C. |
| 7-5/8" | 12" | 3'-0" TO 7'-0" | (2) #5 | (2) #5 | #3 @ 12" O.C. |
| 7-5/8" | 16" | 7'-0" TO 12'-0" | (2) #5 | (2) #5 | #3 @ 12" O.C. |
| 11-5/8" | 7-5/8" | UP TO 7'-0" | (2) #4 | (2) #4 | #3 @ 12" O.C. |
| 11-5/8" | 12" | 7'-0" TO 9'-0" | (2) #5 | (2) #5 | #3 @ 12" O.C. |
| 11-5/8" | 16" | 9'-0" TO 10'-0" | (2) #5 | (2) #5 | #3 @ 12" O.C. |

PROVIDE 7-5/8" MINIMUM BEARING AT EACH SIDE OF OPENING



Revisions:
 1. COORDINATION 09/13/2016
 2.
 3.
 4.
 5.

Project Number
160412

1066 U.S. HIGHWAY 1
New Retail Building - Goodwill
1066 U.S. HIGHWAY 1
Vero Beach, Florida

Project Name
New Retail Building

Stephen Brasgalla, Architect

State Of Florida
Registration No. AR12239
6991 West Broward Boulevard
Suite 100
Plantation, Florida 33317
Telephone 954.614.3801
Telefax 954.208.0600
architect@design23.net

Drawn By: R.S.S. Checked By: S.T.B.

Scale: SHOWN Date: 7-13-16

Project Number
160412

Sheet
S-6.1

PERMIT SET 07-29-2016