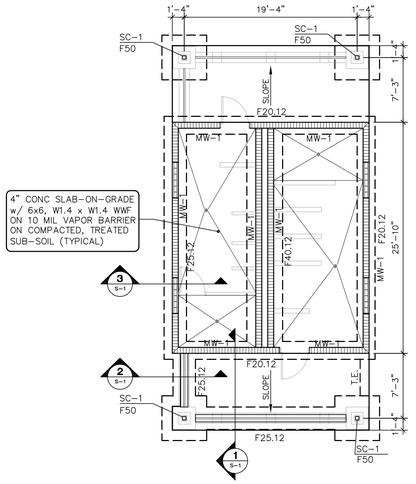
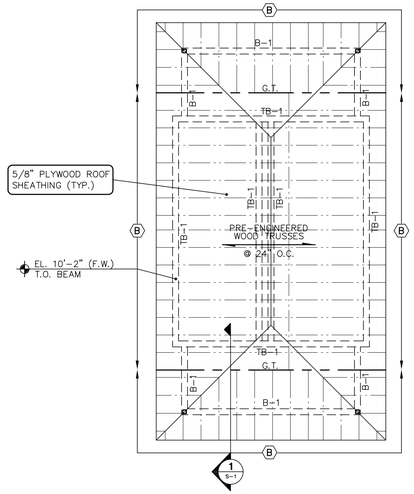


RESTROOM FOUNDATION PLAN

1/8"=1'-0"



RESTROOM ROOF FRAMING PLAN

1/8"=1'-0"

FOUNDATION NOTES:

1. MW-1 MASONRY WALLS SHALL BE 8" CMU w/ #5 @ 48"o.c. U.N.O.
2. ● DENOTES ADDITIONAL #5 VERTICAL BAR.
3. PROVIDE #5 VERTICAL @ ALL JAMBS, CORNERS AND WALL INTERSECTIONS.
4. TOP OF ALL FOOTINGS SHALL BE 1'-4" BELOW TOP OF SLAB.

MASONRY WALL NOTES:

1. WALL SEGMENTS SHALL BE REINFORCED WITH 9 GA. GALVANIZED LATERAL REINFORCING @ 16" O.C. HORIZ. EXTEND REINFORCING 6" INTO POURED ELEMENTS AND AROUND ENCASED STEEL.
2. ADJACENT TO ANY EXTERIOR/INTERIOR WALL OPENING, PLACE (1) MATCHING VERTICAL IN CELL GROUTED SOLID, FULL HEIGHT.
3. ALL MASONRY REINFORCED CELLS SHALL BE FILLED WITH 3000 PSI GROUT MIX.
4. AT END, CORNERS, AND INTERSECTION OF WALLS PLACE (1) MATCHING VERTICAL IN CELL GROUTED SOLID, FULL HEIGHT.

BEAM SCHEDULE

B-1 8"x10" w/ (4)#5 w/ #3 STIRRUPS @ 4" o.c.

FOOTING SCHEDULE

F20.12 2'-0" x 12" x CONT. W/ (2) #5 CONT. BOTT. (1) #5 CONT. TOP

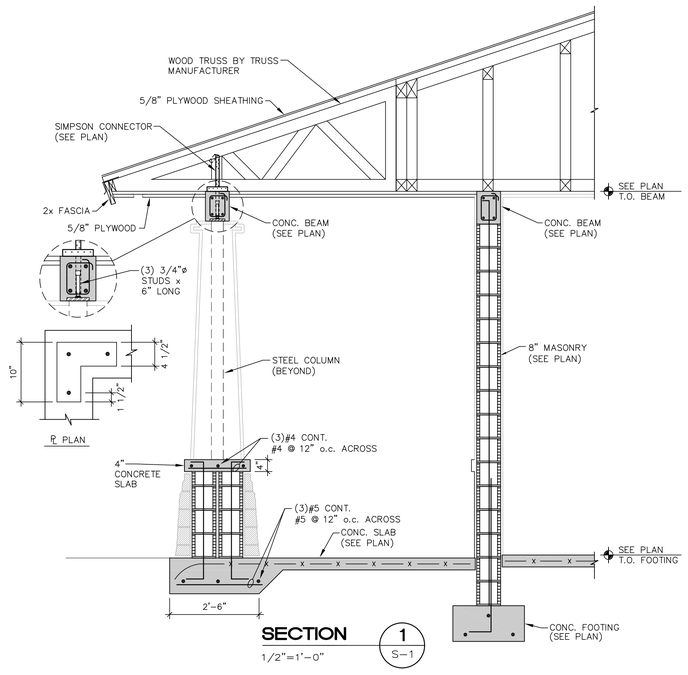
F25.12 2'-6" x 12" x CONT. w/ (5) #5 CONT. (1) TOP & (4) BOTTOM

F40.12 4'-0" x 12" x CONT.

F50 5'-0" x 5'-0" x 12" W/ (5) #5 TOP & BOTTOM EACH WAY

COLUMN SCHEDULE

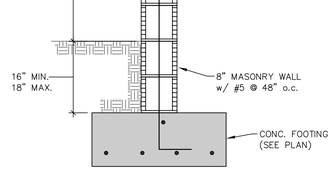
SC-1 HSS 3 1/2" x 3 1/2" x 1/4 w/ 3/4"x10"x0'-10" BASE PLATE w/ (4) 3/4"ø ANCHOR BOLTS



SECTION 1

S-1

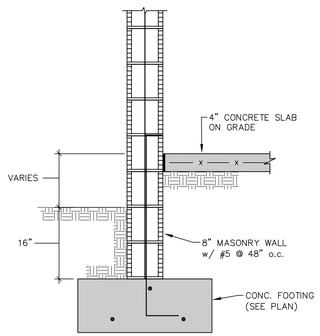
1/2"=1'-0"



SECTION 2

S-1

3/4"=1'-0"



SECTION 3

S-1

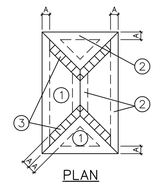
3/4"=1'-0"

PLYWOOD SHEATHING NAILING SCHEDULE

(WALLS AND ROOF)

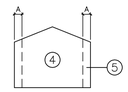
NAIL SIZE	NAIL SPACING	ZONE
8d RING SHANK	4" @ EDGES, 6" @ INTERMEDIATE SUPPORTS	ROOF ③
8d RING SHANK	6" @ EDGES, 6" @ INTERMEDIATE SUPPORTS	ROOF ① ②
8d RING SHANK	4" @ EDGES, 8" @ INTERMEDIATE SUPPORTS	WALL ⑤
8d RING SHANK	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	WALL ④

CORNER DISTANCE, A = X FEET

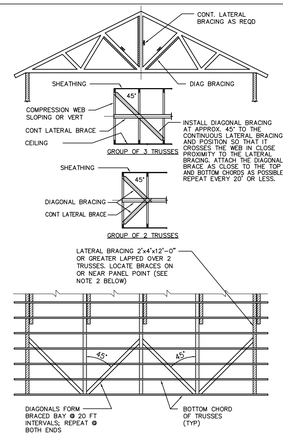


PLAN

HIP ROOF ⊖ ≥ 10° ≤ 45°



ELEVATION



1. WOOD TRUSSES SHALL BE BRACED AND ERECTED IN ACCORDANCE WITH THE "TRUSS PLATE INSTITUTE" AND BCSI: GUIDE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF TRUSSES. BRACING TO BE INSTALLED IN THE PLAN OF THE WEB MEMBERS.
   a. THE TRUSS FABRICATOR SHALL PROVIDE AND LOCATE CONTINUOUS LATERAL BRACING FOR EACH TRUSS WEB MEMBER AS REQUIRED.
   b. LATERAL BRACING SHALL BE RESTRAINED BY DIAGONAL BRACING (MIN. 2" THICK NOMINAL LUMBER). THIS BRACING IS TO BE CONTINUOUS.
   c. A MINIMUM OF TWO ROWS OF DIAGONAL BRACING IS REQUIRED, ONE AT EACH VERTICAL WEB MEMBER CLOSEST TO BEARING LOCATIONS.
2. THE BOTTOM CHORDS SHALL BE BRACED BY CONTINUOUS LATERAL BRACING SPACED AT 8'-0" O. C. WITH A CEILING ATTACHED TO BOTTOM OF TRUSSES. OR IF NO CEILING IS ATTACHED TO BOTTOM OF TRUSSES BRACING SHALL BE MIN. 2 x 4 @ 36" O.C. NAILED TO THE TOP OF THE BOTTOM CHORD. SECURE BRACING TO BOTTOM CHORD W/ (2) 10d x 3" NAILS IF USING 2x4'S AND (3) 10d x 3" NAILS IF USING 2x6'S (TYP.). DIAGONALS PLACED AT 45° TO THE LATERAL BRACES SHALL BE LOCATED AT EACH END. AND AT 20 FOOT INTERVALS IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.
3. TOP CHORD BRACING :
   a. IF PLYWOOD DECKING IS APPLIED DIRECTLY TO TOP CHORD, PROPERLY LAPPED AND NAILED TO DEVELOP DIAPRAGHM ACTION, BRACING IS NOT REQUIRED.
   b. IF PURLINS ARE USED, DIAGONAL TOP CHORD BRACING IS REQUIRED AT EACH END. IF BUILDING EXCEEDS 80 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.

WOOD TRUSS BRACING DETAIL

ULTIMATE

COMPONENT & CLADDING WIND DESIGN PRESSURES

PRESSURES BASED ON V_{ult}	ROOF WIND LOADS			WALL WIND LOADS (SEE NOTE 1)	
	ROOF AREA (10 SF)			WALL AREA (10 SF)	
Kd IS INCLUDED	1	2	3	4	5
PRESSURE (PSF)	36.0	36.0	36.0	63.0	63.0
SUCTION (PSF)	−58.0	−100.0	−100.0	−68.0	−84.0

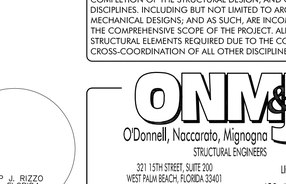
1. EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 5TH EDITION (2014) BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.
2. WIND DESIGN PRESSURES NOTED MAY BE MULTIPLIED BY (.6) FOR COMPARISON TO ALLOWABLE (NOMINAL) WIND PRESSURES OF TESTED ASSEMBLIES. PER SECTION 1609.1.5 OF FBC 5TH EDITION (2014).
3. REFER TO STRUCTURAL NOTES FOR ALL WIND LOAD PARAMETERS.
4. CORNER DISTANCE, A = 3 FEET

ALLOWABLE

COMPONENT & CLADDING WIND DESIGN PRESSURES

PRESSURES BASED ON V_{ult}	ROOF WIND LOADS			WALL WIND LOADS (SEE NOTE 1)	
	ROOF AREA (10 SF)			WALL AREA (10 SF)	
Kd IS INCLUDED	1	2	3	4	5
PRESSURE (PSF)	22	22	22	38.0	38.0
SUCTION (PSF)	−35.0	−60.0	−60.0	−41.0	−51.0

1. EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 5TH EDITION (2014) BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.
2. REFER TO STRUCTURAL NOTES FOR ALL WIND LOAD PARAMETERS.
3. CORNER DISTANCE, A = 3 FEET

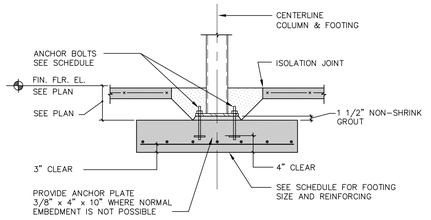


PROVIDE ANCHOR STRAP FROM TABLE BELOW AT EACH BEARING POINT FOR EACH WOOD TRUSS AND EACH GIRDER TRUSS ADEQUATE TO RESIST UPLIFT AS SPECIFIED BY THE WOOD TRUSS MANUFACTURER.

TRUSS TIE DOWN SCHEDULE

MARK	MANUF & MODEL NO.	STRUCTURAL COMPONENT	PRODUCT APPROVAL	NAILS TO COMPONENT	NAILS TO SEAT	BOLTS	ALLOWABLE UPLIFT	LATERAL LOAD PARALLEL TO WALL	LATERAL LOAD PERPEND. TO WALL
(A)	SIMPSON H1T-L12	TRUSS/MASONRY OR CONCRETE	FL11473.5	7-10d x 1 1/2"	–	–	1265 # (*)	415 #	1100 #
(B)	SIMPSON H1T-L16	TRUSS/MASONRY OR CONCRETE	FL11473.5	14-10d x 1 1/2"	5-10d x 1 1/2"	–	1810 # (*)	415 #	1100 #
(C)	SIMPSON MG7	GIRDER TRUSS/ MASONRY OR CONC.	FL11470.7	22-10d	NOTE 5	5/8"ø	3965 # (**)	–	–
(D)									
(F)									
(F)	SIMPSON ST6224	–	FL10852.13	14-16d TO TRUSS	14-16d TO BEAM	–	2540 # (**)	–	–
(G)	SIMPSON MTS 12	–	FL10456.33	7-10d x 1 1/2"	7-10d x 1 1/2"	–	840 # (*)	–	–
(H)	SIMPSON MTS 12	–	FL10456.33	7-10d	7-10d	–	1000 # (*)	–	–
(I)	SIMPSON RHETA 20	TRUSS/MASONRY OR CONCRETE	FL11473.9	12-10dx1 1/2"	–	–	2235 # (*)	335 #	730 #
(J)	SIMPSON HGT-2	GIRDER TRUSS/ MASONRY OR CONC.	FL10456.18	16-10d	NOTE 5	2-3/4"ø	10980 # (**)	–	–
(K)	HGAM 10	TRUSS / CMU	FL11473.6	4-SDS 1/4"x 1 1/2"	(4)1/4" X 2 3/4" TITEN	–	850 #	1105 #	1005 #
(L)	SIMPSON LGT2	–	FL11470.6	16-16d SINKER	(7)1/4" X 2 1/4" TITEN	–	2150 # (**)	–	–
(M)	SIMPSON MTSM 16	–	FL11473.2	7-10d	(4)1/4" X 2 1/4" TITEN TO BEAM	–	860 # (*)	235 #	190 #

NOTES:
1. (*) – ONE PLY MEMBER (**) – TWO PLY (MIN.) MEMBER
2. T.B. – THRU-BOLT
3. U.N.O. – UNLESS NOTED OTHERWISE
4. APPROVED EQUAL OR BETTER TIE DOWNS FOR THE SAME LATERAL & UPLIFT LOADS ARE ACCEPTABLE.
5. USE "ULTRABOND 1" EPOXY W/ 12" MIN. EMBED.
6. USE "ULTRABOND 1" EPOXY W/ 5" MIN. EMBED.



TYPICAL STEEL COLUMN FOOTING

N.T.S.

NOT FOR CONSTRUCTION

DESIGN DEVELOPMENT

6/30/16

THESE ARE PROGRESS DRAWINGS, THEREFORE THEY ARE ISSUED PRIOR TO COMPLETION OF THE STRUCTURAL DESIGN, AND OTHER DESIGN DISCIPLINES. INCLUDING BUT NOT LIMITED TO ARCHITECTURAL OR MECHANICAL DESIGNS; AND AS SUCH, ARE INCOMPLETE BY NATURE FOR THE COMPREHENSIVE SCOPE OF THE PROJECT. ALLOWANCES FOR STRUCTURAL ELEMENTS REQUIRED DUE TO THE COMPLETION AND CROSS-COORDINATION OF ALL OTHER DISCIPLINES SHOULD BE MADE.

PHILIP J. RIZZO P.E. FLORIDA REG. #60998

Structural Drawing Updated
Based on Architectural Backgrounds Dated 6-13-16

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DATE 06-30-16

SCALE

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S-1