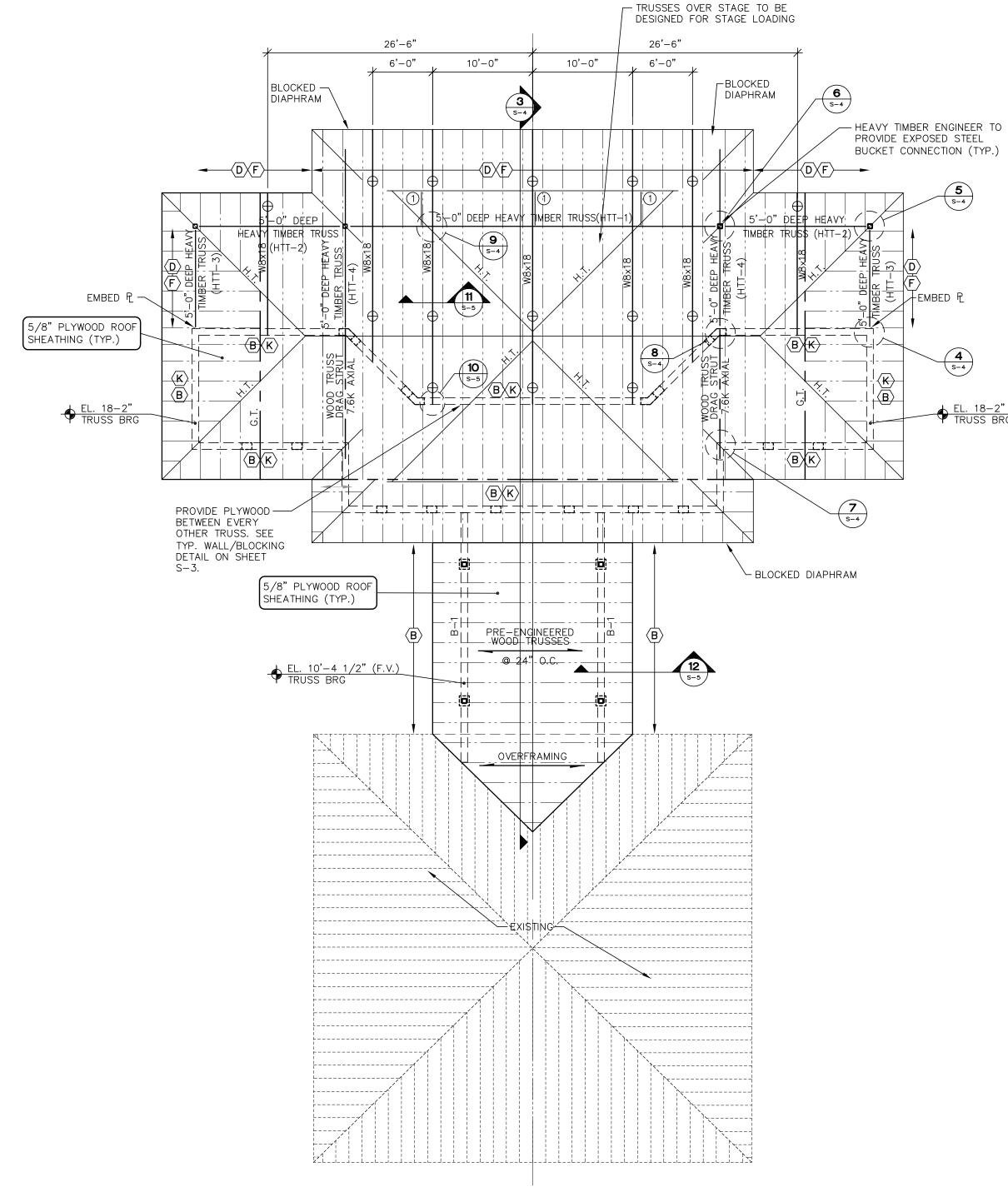


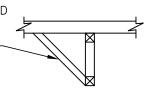
- 1. WOOD TRUSSES SHALL BE BRACED AND ERECTED IN ACCORDANCE WITH THE "TRUSS PLATE INSTITUTE" AND BCSI; GUIDE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF TRUSSES. BRACING TO BE INSTALLED IN THE PLAN OF THE WEB MEMBERS.
- a. THE TRUSS FABRICATOR SHALL PROVIDE AND LOCATE CONTINUOUS LATERAL BRACING FOR EACH TRUSS WEB MEMBER AS REQUIRED.
- b. LATERAL BRACING SHALL BE RESTRAINED BY DIAGONAL BRACING (MIN. 2" THICK NOMINAL LUMBER). THIS BRACING IS TO BE CONTINUOUS.
- c. A MINIMUM OF TWO ROWS OF DIAGONAL BRACING IS REQUIRED, ONE AT EACH VERTICAL WEB MEMBER CLOSEST TO BEARING LOCATIONS.
- 2. THE BOTTOM CHORDS SHALL BE BRACED BY CONTINUOUS LATERAL BRACING SPACED AT 8'-0" O. C. WITH A CEILING ATTACHED TO BOTTOM OF TRUSSES. OR IF NO CEILING IS ATTACHED TO BOTTOM OF TRUSSES BRACING SHALL BE MIN. 2 x 4 @ 36" O.C. NAILED TO THE TOP OF THE BOTTOM CHORD. SECURE BRACING TO BOTTOM CHORD W/ (2) 10d x 3" NAILS IF USING 2x4'S AND (3) 10d x 3" NAILS IF USING 2x6'S (TYP.). DIAGONALS PLACED AT 45" TO THE LATERAL BRACES SHALL BE LOCATED AT EACH END. AND AT 20 FOOT INTERVALS IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.
- 3. TOP CHORD BRACING:
- a. IF PLYWOOD DECKING IS APPLIED DIRECTLY TO TOP CHORD, PROPERLY LAPPED AND NAILED TO DEVELOP DIAPRAGHM ACTION, BRACING IS NOT REQUIRED.
- b. IF PURLINS ARE USED, DIAGONAL TOP CHORD BRACING IS REQUIRED AT EACH END. IF BUILDING EXCEEDS 80 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.

WOOD TRUSS BRACING DETAIL



NOTES:

- 1. PROVIDE PLYWOOD BETWEEN EVERY OTHER TRUSS. SEE TYP. WALL/BLOCKING DETAIL ON SHEET S-3.
- 2. HEAVY TIMBER TRUSSES TO BE DESIGNED BY HEAVY TIMBER ENGINEER.
- → INDICATES 2,000 LB POINT LOAD
- 1 INDICATES 6x6 TIMBER BRACE— TO BOTT. CHORD OF TRUSS
- B-1 8x12" w/ (4)#6 #3 STIRRUPS @ 5" o.c.



ULTIMATE

COMPONENT & CLADDING WIND DESIGN PRESSURE

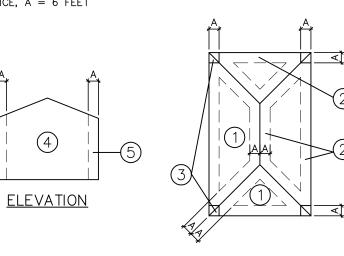
PRESSURES BASED ON VASO	ROC	OF WIND LO	WALL WIND LOADS (SEE NOTE 1)		
	R	OOF AREA (10 SI	WALL AREA (10 SF)		
Kd IS INCLUDED	1	2	3	4	5
PRESSURE (PSF)	40.1	40.1	40.1	69.5	69.5
SUCTION (PSF)	-63.6	-110.8	-110.8	-74.5	-93.1

1. EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 5TH EDITION (2014) B' EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS. 2. WIND DESIGN PRESSURES NOTED MAY BE MULTIPLIED BY (.6) FOR COMPARISON TO ALLOWABLE (NOMINAL) WIND PRESSURES OF TESTED ASSEMBLIES. PER SECTION 1609.1.5 OF FBC 5TH EDITION (2014).

3. REFER TO STRUCTURAL NOTES FOR ALL WIND LOAD PARAMETERS. 4. CORNER DISTANCE, A = 6 FEET

ALLOWABLE								
COMPONENT & CLADDING WIND DESIGN PRESSURES								
PRESSURES BASED ON Volt	ROC	ADS	WALL WIND LOADS (SEE NOTE 1)					
	R	OOF AREA (10 S	WALL AREA (10 SF)					
Kd IS INCLUDED	1	2	3	4	5			
PRESSURE (PSF)	24	24	24	41.7	41.7			
SUCTION (PSF)	-38.2 -66.5		-66.5	45.3	-55.9			

1. EXTERIOR GLAZED OPENINGS IN BUILDINGS SHALL COMPLY WITH FLORIDA BUILDING CODE 5TH EDITION (2014) BY EITHER BEING DESIGNED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS. 2. REFER TO STRUCTURAL NOTES FOR ALL WIND LOAD PARAMETERS. 3. CORNER DISTANCE, A = 6 FEET



	PROVIDE ANCHOR STRAP FROM TABLE BELOW AT EACH BEARING POINT FOR EACH WOOD TRUSS AND EACH GIRDER TRUSS ADEQUATE TO RESIST UPLIFT AS SPECIFIED BY THE WOOD TRUSS MANUFACTURER.										
TRUSS TIE DOWN SCHEDULE											
MARK	MANUF & MODEL NO.	STRUCTURAL COMPONENT	PRODUCT APPROVAL	NAILS TO COMPONENT	NAILS TO SEAT	BOLTS	ALLOWABLE UPLIFT	LATERAL LOAD PARALLEL TO WALL	LATERAL LOAD PERPEND. TO WALL		
(A)	SIMPSON HETAL 12	TRUSS/MASONRY OR CONCRETE	FL11473.5	7-10d x 1 1/2"	-	1	1265 # (*)	415 #	1100 #		
(B)	SIMPSON HETAL 16	TRUSS/MASONRY OR CONCRETE	FL11473.5	14-10d x 1 1/2"	5-10d x 1 1/2"	-	1810 # (*)	415 #	1100 #		
(c)	SIMPSON MGT	GIRDER TRUSS/ MASONRY OR CONC.	FL11470.7	22-10d	NOTE 5	5/8 " ø	3965 # (**)	-	-		
(D)	SIMPSON H14	TRUSS		12-8d x 1 1/2"			1350 #	265 #	515 #		
F	SIMPSON HGA 10	TRUSS		4-SDS 1/4" x 3"			695 #	940 #	1165 #		
F	SIMPSON ST6224	_	FL10852.13	14-16d TO TRUSS	14-16d TO BEAM	-	2540 # (**)	-	-		
(G)	SIMPSON MTS 12	-	FL10456.33	7-10d x 1 1/2"	7-10d x 1 1/2"	-	840 # (*)	-	-		
$\langle H \rangle$	SIMPSON MTS 12	_	FL10456.33	7–10d	7–10d	-	1000 # (*)	_	-		
	CILIDOCOLI	TOUCC /MACONDY							1		

16-10d

16-16d SINKER 7-10d

2. T.B. - THRU-BOLT

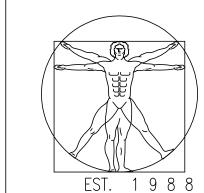
4. APPROVED EQUAL OR BETTER TIE DOWNS FOR THE SAME LATERAL & UPLIFT LOADS ARE ACCEPTABLE.

FL10456.18

FL11470.6

5. USE "ULTRABOND 1" EPOXY W/ 12" MIN. EMBED. 6. USE "ULTRABOND 1" EPOXY W/ 5" MIN. EMBED.

 \circ P L A N N E R S $I\ N\ C\ O\ R\ P\ O\ R\ A\ T\ E\ D$



RICK GONZALEZ, A.I.A. PRESIDENT AR - 0014172

300 CLEMATIS STREET WEST PALM BEACH FLORIDA PH: (5 6 1) - 6 5 9 - 2 3 8 3 FAX: (5 6 1) - 6 5 9 - 5 5 4 6

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AMPHITHEATRE COMMONS PARK

E TO KESIST OF EILT AS SPECIFIED BY THE WOOD TROSS MANOTACTOREK.													
RUSS TIE DOWN SCHEDULE													
	PRODUCT APPROVAL	NAILS TO COMPONENT	NAILS TO SEAT	BOLTS	ALLOWABLE UPLIFT	LATERAL LOAD PARALLEL TO WALL	LATERAL LOAD PERPEND. TO WALL	R	11600 POINCIANA BLVD. ROYAL PALM BEACH, FL 3341				
Υ	FL11473.5	7-10d x 1 1/2"	-	-	1265 # (*)	415 #	1100 #						
Υ	FL11473.5	14-10d x 1 1/2"	5-10d x 1 1/2"	-	1810 # (*)	415 #	1100 #						
).	FL11470.7	22-10d	NOTE 5	5/8 " ø	3965 # (**)	-	-						
		12-8d x 1 1/2"			1350 #	265 #	515 #						
		4-SDS 1/4" x 3"			695 #	940 #	1165 #						
	FL10852.13	14-16d TO TRUSS	14-16d TO BEAM	-	2540 # (**)	1	-						
	FL10456.33	7-10d x 1 1/2"	7-10d x 1 1/2"	-	840 # (*)	-	-	R	EVISIONS		NO		
	FL10456.33	7–10d	7–10d	-	1000 # (*)	-	_	-			_		
Υ	FL11473.9	12-10dx1 1/2"	-	-	2235 # (*)	335 #	730 #				$ / \rangle$		

2150 # (**)

860 # (*) 235 #

850 # 1105 # 1005 #

190 #

DATE

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NOTE 5 2-3/4"ø 10980 # (**)

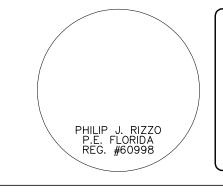
1. (*) - ONE PLY MEMBER (**) - TWO PLY (MIN.) MEMBER

(K) | HGAM 10 | TRUSS / CMU | FL11473.6 | 4-SDS 1/4"x 1 1/2"

3. U.N.O. - UNLESS NOTED OTHERWISE

NOT FOR CONSTRUCTION

igl(these are progress drawings, therefore they are issued prior to COMPLETION OF THE STRUCTURAL DESIGN, AND OTHER DESIGN DISCIPLINES. INCLUDING BUT NOT LIMITED TO ARCHITECTURAL OR MECHANICAL DESIGNS; AND AS SUCH, ARE INCOMPLETE BY NATURE FOR THE COMPREHENSIVE SCOPE OF THE PROJECT. ALLOWANCES FOR STRUCTURAL ELEMENTS REQUIRED DUE TO THE COMPLETION AND CROSS-COORDINATION OF ALL OTHER DISCIPLINES SHOULD BE MADE.





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