

EXHIBIT "B"
SOIL REPORT
W/ENGINEERED FILL &
FOUNDATION RECOMMENDATION
KELSEY RESTROOM BUILDING

Terracon

December 21, 2015

CPZ Architects, Inc.
4316 West Broward Boulevard
Plantation, Florida 33317

Attention: Mr. Erik Garcia, AIA

RE: Subsurface Study
Kelsey Park Restroom Facility
Lake Park, Florida
Project Number: 34155075

Gentlemen:

1.0 INTRODUCTION

Terracon Consultants, Inc. (Terracon) has completed a subsurface study in connection with the new restroom facility to be constructed at Kelsey Park in Lake Park, Florida. Results of the study are presented herein.

2.0 PROJECT CONSIDERATIONS

The project will involve the design and construction of a new restroom facility that will be sited about 80 feet northwest of the northwest corner of the existing southern tennis court. The restroom will be one story in height with plan dimensions of roughly 25 feet by 35 feet. We assume that the facility will be of CBS construction and that its perimeter and interior walls will support the roof system. For purposes of foundation evaluation, we have assumed that the walls will impose a maximum load of 3000 pounds per lineal foot on their foundations.

3.0 SURFACE FEATURES OF SITE

The site of the new restroom facility is devoid of structures and covered with maintained grass. Topography within the area is flat and featureless. Ground surface elevations are not known to us.

4.0 SURFACE CONDITIONS

Subsurface conditions with the area of the new structure were explored with two borings located as described on the following page.

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Environmental

Facilities

Geotechnical

Materials

Boring Designation

Location

| | |
|------|--|
| AB-1 | 95 feet west and 32 feet north of control point. |
| AB-2 | 60 feet west and 5 feet north of control point. |

Note: Control point is the northwest corner of the southern tennis court.

The borings were accomplished manually using a three inch diameter bucket auger and extended to a depth of 6.5 feet below grade. As the borings were advanced, cone penetrometer soundings were conducted at one foot vertical intervals to assess the shear strength and compressibility of the profile components. A Brainard-Kilman Model S-214 device was used for this purpose.

Logs which describe the materials encountered in the borings along with results of the cone penetrometer tests are furnished on Sheets 1A and 1B, attached. Reference to the logs shows that the borings chiefly encountered loose to medium dense clean to slightly silty sands beneath a six inch thick veneer of topsoil. Of note, a six inch thick layer of organic fine sand was found in the soil profile of Boring AB-1 between depths of 2.5 and 3 feet below grade.

Groundwater levels were measured in the boreholes when the free water surface was initially intercepted. The depth to the water table was 2.0 feet in Boring AB-1 and 2.3 feet in Boring AB-2 on the day of the field exploration, which was December 11, 2015. We expect the water table level to be near the wet season high.

5.0 LABORATORY TESTING

A sample of the organic sand was tested for natural moisture content (ASTM D 2216) and organic content (ASTM D 2974). The test yielded a natural moisture content value of 32.2 percent and an organic content of 6.5 percent. By local engineering convention, soils with an organic content of more than 5 percent are considered organic in composition.

6.0 GEOTECHNICAL EVALUATION

Results of the study indicate that the site is suitable for the planned construction when reviewed from a geotechnical engineering perspective. Following conventional site preparation, the structure may be supported on a shallow foundation system and employ a slab-on-grade for its ground floor.

7.0 CLEARING, GRUBBING AND STRIPPING

Surface vegetation should be cleared from the structure area and its root system grubbed from the soils. Clearing and grubbing should consist of the complete removal and disposal of timber, brush, stumps, roots, rubbish, debris, asphalt and all other obstructions resting on or protruding through the surface of the existing ground and the surface of excavated areas. All roots greater than one inch in diameter, or high concentrations of smaller diameter roots exposed by clearing, grubbing and stripping, should be removed to a depth of not less than 12 inches.

Topsoil that covers the site should be removed from the structure footprint. If not required for landscaping, these materials should be wasted.

8.0 ENGINEERED FILL

Fill required to bring the building area to construction grade should consist of clean granular materials that are free of debris, cinders, combustibles, roots, sod wood, cellulose and organic material. It should have not more than 12 percent passing the U.S. Standard No. 200 Sieve (dry weight basis), no particle size larger than two inches and not more than two percent organics by weight.

The fill should be placed at a water content within two percent of optimum in lifts that do not exceed 12 inches in loose thickness. Each lift should be thoroughly and uniformly compacted to attain 95 percent of the ASTM D 1557 maximum dry density.

9.0 FOUNDATION DESIGN AND CONSTRUCTION

The proposed structure may be safely supported on a monolithic thickened edge slab foundation that is based in the improved native soils and/or engineered and proportioned for a net allowable bearing pressure of 2500 pounds per square foot. To provide a satisfactory factor of safety against a shearing failure in the supporting soils: (1) the footing bottoms should bear at least 12 inches below adjacent grade; (2) continuous footings should be at least 16 inches wide and (3) isolated foundations, if used, should have a width not less than 24 inches.

Foundation subgrades to at least 12 inches beneath the footing bearing levels should be compacted to 95 percent of the ASTM D 1557 maximum dry density prior to placing formwork and reinforcing steel for the substructure. Excavations for the load bearing elements should be maintained in a dry condition while awaiting placement of reinforcing steel and concrete.

Foundations designed and constructed in the recommended manner are estimated to sustain a maximum total settlement of somewhat more than one half inch provided that the wall loadings do not exceed 3,000 pounds per lineal foot. Differential settlement that occurs between adjacent foundations should not be more than one quarter inch. Angular distortion along the

alignments of the bearing walls that is produced by differential settlement is estimated to be less than 1 in 500.

10.0 GROUND FLOOR SLAB

Slab-on-grade construction may be used for the ground floor of the structure. The concrete of the slab should be cast upon granular soils that are compacted to not less than 95 percent of the ASTM D 1557 maximum dry density.

An impervious membrane should be installed between the underside of the floor slab and the soil substrate to serve as a barrier to moisture rise from the subgrade. Ordinarily, a 6-mil thick film of polyethylene is sufficient for this purpose. However, floor coverings that are particularly sensitive to moisture flux may require a thicker membrane and/or one of different composition.

11.0 GENERAL COMMENTS

The conclusions and recommendations presented herein are based upon the information obtained from the borings performed at the indicated locations and other information discussed in the report. The report does not reflect any variations that may occur between the borings or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until, during or after construction. If variations appear, we should be immediately notified so that further evaluation and supplemental recommendations can be provided if warranted.

The scope of our services did not include, either specifically or by implication, any environmental or biological (e.g. mold, fungi and bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials, or conditions. Should there be a concern about the potential for such contamination or pollution, other studies should be undertaken.

This report has been prepared for the exclusive use of our client for specific application to the project discussed herein and has been prepared in accordance with generally accepted geotechnical engineering practices. No warranties, either expressed or implied, are intended or made.

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We trust that the information provided in the report is clear and understandable. Should it require any clarification or amplification, feel free to contact Thomas J. Tepper, P.E. at 954-703-1839.

Subsurface Study
Kelsey Park Restroom Facility
Lake Park, Florida
Project Number: 34155075 ■ December 21, 2015

Terracon

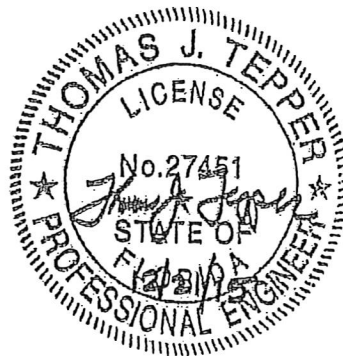
Very truly yours,

Terracon Consultants, Inc.

RN
Rutu Nulkar, P.E.
Project Engineer
FL Registration No. 70625

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Thomas J. Tepper, P.E.
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Attachments: Sheet 1A – Log of Boring AB-1
Sheet 1B – Log of Boring AB-2



| <i>Log of Auger Boring</i> | | | |
|----------------------------|--|-----------------------------|----------|
| Project Number: | 34155075 | Boring Number: | AB-1 |
| Client: | CPZ Architects, Inc. | | |
| Project: | Kelsey Park Restroom Facility Lake Park, Florida | | |
| Boring Location: | 95 feet west and 32 feet north of northwest corner of south tennis court | | |
| Date Drilled: | 12-11-15 | Drilled By: | RR |
| Ground Elevation: | Not determined | Depth of Groundwater Table: | 2.0 feet |

| <i>Depth (feet)</i> | <i>Soil Type</i> | <i>Soil Description</i> |
|---------------------|------------------|--|
| 0.0-0.5 | SM | Dark gray silty fine SAND with grass roots (Topsoil) |
| 0.5-1.5 | SP | Dark brown fine to medium SAND |
| 1.5-2.5 | SP | Light brown fine to medium SAND, trace shell and limestone fragments |
| 2.5-3.0 | OL-SM | Black slightly silty organic fine SAND |
| 3.0-4.0 | SM | Dark gray slightly silty fine to medium SAND |
| 4.0-5.0 | SP | Gray fine to medium SAND, trace shell fragments |
| 5.0-6.5 | SP | Brown fine to medium SAND |
| | | |
| | | |

| <i>CONE PENETRATION TEST RESULTS</i> | |
|--------------------------------------|--|
| <i>Depth (feet)</i> | <i>Cone Penetration Resistance (tsf)</i> |
| 0-1 | 50 |
| 1-2 | 50 |
| 2-3 | 50 |
| 3-4 | 50 |
| 4-5 | 50 |
| 5-6 | 50 |
| | |
| | |
| | |
| Total Depth of Boring: 6.5 feet | |

*Cone penetrometer resistance determined using a Brainard-Kilman Model S-214 hand-held static cone penetrometer.

| <i>Log of Auger Boring</i> | | |
|----------------------------|---|--------------------------------------|
| Project Number: | 34155075 | Boring Number: AB-2 |
| Client: | CPZ Architects, Inc. | |
| Project: | Kelsey Park Restroom Facility Lake Park, Florida | |
| Boring Location: | 60 feet west and 5 feet north of northwest corner of south tennis court | |
| Date Drilled: | 12-11-15 | Drilled By: RR |
| Ground Elevation: | Not determined | Depth of Groundwater Table: 2.3 feet |

| <i>Depth (feet)</i> | <i>Soil Type</i> | <i>Soil Description</i> |
|---------------------|------------------|--|
| 0.0-0.5 | SM | Dark gray silty fine SAND with grass roots |
| 0.5-1.0 | SP | Dark brown fine to medium SAND |
| 1.0-2.5 | SM | Dark gray slightly silty fine to medium SAND |
| 2.5-3.5 | SM | Dark gray slightly silty fine to medium SAND, trace fine roots |
| 3.5-4.5 | SP | Brown fine to medium SAND, trace fine roots |
| 4.5-5.5 | SP | Light brown fine to medium SAND |
| 5.5-6.5 | SP | Brown fine to medium SAND |
| | | |
| | | |

| <i>CONE PENETRATION TEST RESULTS</i> | |
|--------------------------------------|--|
| <i>Depth (feet)</i> | <i>Cone Penetration Resistance (tsf)</i> |
| 0-1 | 15 |
| 1-2 | 30 |
| 2-3 | 35 |
| 3-4 | 35 |
| 4-5 | 35 |
| 5-6 | 35 |
| | |
| | |
| | |
| Total Depth of Boring: 6.5 feet | |

*Cone penetrometer resistance determined using a Brainard-Kilman Model S-214 hand-held static cone penetrometer.

EXHIBIT "C"
SCHEDULE OF PARK EVENTS
KELSEY & LAKE SHORE PARKS

KELSEY PARK

April 2017 - Easter Egg Hunt
May 2017 – Memorial Day Ceremony
June-August – Summer Camp Activities
September 2017 – Labor Day Ceremony

LAKE SHORE PARK

Re-occurring every Monday night---- Dancing at Community Meeting Room
Monthly Ladies Luncheon at Community Meeting Room
June-August – Summer Camp Activities
July - Independence Day Celebration